## VARIANCE APPLICATION

**Planning & Zoning** 2635 Talley Street Decatur, GA 30030 Phone 404-377-6198 Fax 404-378-5054



Attach a survey of the property drawn to scale and showing the following information. Please provide one full-size copy of all plans, as well as one copy of all plans in an 8<sup>1</sup>/<sub>2</sub> x 11<sup>\*</sup> format. It is helpful to show floor plans and elevations of proposed building improvements, as well as a letter of support from adjacent property owners. If the applicant is not the current property owner, provide a notarized authorization for this application from the current property owner. See the stream variance submittal checklist for additional requirements for stream variances.

- 1. all property lines with dimensions
- 2. location of buildings and other structures, creeks and easements referenced to property lines
- 3. north arrow, scale, lot and block numbers and land lot
- 4. topographic and drainage information if pertinent

Address of property 906 N PARKWOOD ROA	0 Decatur, GA 30030
Name of applicant RAMA DESIGN BULD LLC (ZALHARY BRANCH)	Phone 503 758 0970
Address 480 MELLVIEW AVE SW	City/state/ZIP ATLANTA GA 30310
Email RAMA DESIGNBUILDGGMAIL, COM	
JOHN NATHANIEL GREENE + Name of property owner <u>SARAH FRIEDENTHAL-GREENE</u>	Phone 917 821 0376
Address 906 N PARKWOOD ROAD	City/state/ZIP DELATUR 6A 30030
Current zoning of property R-60	

Please answer all of the following questions on a separate sheet.

- 1. What is the variance requested? What code requirement do you wish to vary from?
- 2. What are the special conditions relating to the specific piece of property in question (narrowness, shallowness, shape, topography, or other extraordinary and exceptional situation)?
- 3. Explain how the application of the zoning ordinance to this specific piece of property results in peculiar, extraordinary and practical difficulties?
- 4. Are the circumstances or conditions applying to the building or land in question peculiar to the premises? Do they apply generally to other land or buildings in the vicinity?
- 5. Explain why the granting of this variance is necessary for the preservation and enjoyment of a property right and does not merely serve as a convenience to the applicant.
- 6. Did the condition for which the variance is sought result from an action by the applicant?
- 7. Explain how the variance will affect the supply of light and air to adjacent property, the traffic on public streets, the danger of fire, the public safety and established property values.
- 8. Explain how the granting of the variance will be in harmony with the general purpose and intent of the Decatur land use plan.
- 9. Will the granting of the variance allow a structure or use in a district restricted against such structure or use?

I hereby certify that the above and attached statements and documents are true to the best of my knowledge and belief.

Applicant signature _	m	Date	4/5/24	

### **Variance Application Questions**

906 N Parkwood Road Decatur GA 30030

#### Permit #... 24DEC-RA00058

- 1. The existing house and driveway fall within the setbacks of the property. The proposed driveway and 2nd story addition to the house will not expand on the footprint of the existing driveway or house. The driveway will be replaced with the exact same size, and the 2nd story addition will be built plumb from the existing exterior walls.
- 2. Narrowness / shape

3. The footprint of the existing house and driveway will not change with our proposed work.

4. Both the adjacent properties also have encroachments to their sides of the setbacks.

5. The homeowners have a growing family and their current house will not sustain their growing needs. Since the footprint of the existing house and driveway will not change with our proposed work, and the neighbors are in support, said work would fall within "the preservation and enjoyment of a property right".

6. No, nothing has changed since the current homeowners bought the property.

7. Since the proposed 2nd story addition will be only on the back 1/3 of the house (west facing), there would not be a significant affect on the supply of light and air to adjacent properties. The traffic, danger of fire, and public safety will not change. The established property value would increase.

8. The granting of this variance will provide a longtime local Decatur family with the space in their home to accommodate their growing family.

9. No, it would not.



FLOOD NOTE:

IS SUGGESTED FOR MORE ACCURATE INFORMATION. FOR FURTHER INFORMATION CONTACT THE LOCAL DRAINAGE DEPARTMENT, CORPS OF ENGINEERS AND INSURANCE COMPANY OR AN APPRAISER.

#### SURVEY NOTES:

BEEN PAYED OR COVERED OVER. THE LOCATION OF UNDERGROUND UTILITIES BEEN PAYED OR COVERED OVER. THE LOCATION OF UNDERGROUND UTILITIES AS SHOWN HEREON ARE BASED ON ABOVE GROUND STRUCTURES AND RECORD DRAWINGS PROVIDED TO THE SURVEYOR. LOCATION OF UNDERGROUND MAY VARY FROM LOCATIONS SHOWN HEREON. ADDITIONAL BURIED UTILITIES MAY BE ENCOUNTERED. NO EXCAVATIONS WERE MADE DURING THE PROCESS OF THIS SURVEY TO LOCATE BURIED UTILITIES. BEFORE EXCAVATIONS ARE BEGUN, TELEPHONE, ELECTRIC, WATER AND SEWER, GAS COMPANIES SHOULD BE CONTACTED FOR VERIFICATION OF UTILITY TYPE AND FOR FIELD LOCATIONS. 2. THIS PLAT WAS PREPARED TO SHOW THE APPROXIMATE LOCATION OF THE IMPROVEMENTS AND IS NOT RECORDABLE. FENCES SHOULD NOT BE LOCATED USING SIDE DIMENSIONS FROM THE HOUSE. ALL MATTERS OF THE TITLE ARE EXCEPTED. THIS PLAT IS SUBJECT TO ALL LEGAL EASEMENTS AND RIGHT OF WAY PUBLIC OR PRIVATE.

CLIENT OR USER OF THIS DATA VERIFY THIS INFORMATION WITH THE ISSUINCE AUTHORITY

NOT BE SCANNED AND ALTERED, CROPPED OUT COPY/PASTE OR MODIFIED WITH SURVEY LAND EXPRESS TITLE BLOCK, SURVEYOR'S STAMP AND SIGNATURE. THIS SURVEY PLAT CAN BE ONLY ATTACHED AS A SEPARATE DOCUMENT BY ITSELF TO

#### ZONING NOTE:

DEVELOPER AND ARCHITECT TO CONFIRM ZONING DISTRICT, PER ZONING DEPARTMENT. ZONING INFORMATION SHOWN HEREON WAS DERIVED FROM ONLINE SOURCE MUNICODE.



### (APP) R.R 10"HW 5"HW LINE Q Alf // 15"HW 60.00'<sub>(F=</sub> SEABOARD, RAIL ROAD 4"H 20"H ΉW









PROPOSED	LOT	COV	'ERAGE:
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EXISTING HOUSE:	1783 SF
EXISTING PATIO:	570 SF
EXISTING WALLS:	69 SF
EXISTING GARAGE:	243 SF
EXISTING DRIVEWAY:	868 SF
EXISTING FRONT PORCH:	76 SF
SUB-TOTAL:	3609 SF
PROPOSED STEPS:	233 SF
SUB-TOTAL:	233 SF
TOTAL:	3,842 SF
NEW IMPERVIOUS AREAS = $233$ SF	

REQUIRES NEW STORMWATER MANAGEMENT BMPs

3

RELEASED FOR CONST	RUCTION		MISC. DE	TAILS				
LOT 5			SITE PLAN P	REPARED FOR:	SHEET 3 OF			
RE-SUBDIVISION OF PART OF CHELSE	A HEIGHTS							
LAND LOT 4	18TH DISTRICT			T MEDENTIAL	GIVEENE			
DEKALB COUNTY, GEORGIA	PB.19/PG.151	DB.25980/PG.605	PRO	PERTY ADDRESS:	E O R G			
FIELD WORK DATE MARCH 30, 2023	PRINTED/SIGNED M	IARCH 11, 2024	906	N PARKWOOD ROAD	G REGISTERED Y			
ALL MATTERS PERTAINING TO TITLE AF	ALL MATTERS PERTAINING TO TITLE ARE EXCEPTED		DECATUR, GA 30030		* 19 31070			
THE FIELD DATA UPON WHICH THIS PLAT IS BASED H SQUARES METHOD. THIS PLAT HAS BEEN CALCULAT GATHER THE INFORMATION USED IN THE PREPARATIO	THE FIELD DATA UPON WHICH THIS PLAT IS BASED HAS A CLOSURE OF 1 FOOT IN 30,000+ FEET, AN ANGULAR ERROR OF 05 SECONDS PER ANGLE POINT AND WAS ADJUSTED USING THE LEAST SQUARES METHOD. THIS PLAT HAS BEEN CALCULATED FOR CLOSURE AND FOUND TO BE ACCURATE TO 1 FOOT IN 100,000+ FEET. AN ELECTRONIC TOTAL STATION AND A 100° CHAIN WERE USED TO GATHER THE INFORMATION USED IN THE PREPARATION OF THIS PLAT. NO STATE PLANE COORDINATE MONUMENT FOUND WITHIN 500° OF THIS PROPERTY.							
AU COORD #20230470	SURVEY	LAND EXPRES	S, INC	24 LENOX POINTE ATLANTA, GA 30324 FAX 404–601–0941	A. STEP			
DWG <u>#20230470</u> SP	LAND	SURVEYING SERVICES		TEL 404-252-5747 INFO@SURVEYLANDEXPRESS.COM	IN MY OPINION, THIS PLAT IS A CORRECT REPRESENTATION OF THE LAND PLATED AND HAS BEEN PREPARED IN CONFORMITY WITH THE MINIMUM STANDARDS AND REQUIREMENTS OF LAW.			

# **FRIEDENTHAL-GREENE RESIDENCE**

# **SINGLE FAMILY DWELLING - RENOVATION NATHANIEL & SARAH FRIEDENTHAL-GREENE**



**CODE INFORMATION:** 

## **GENERAL NOTES:**

- G.C. SHALL VISIT JOB SITE AND BECOME FAMILIAR WITH EXISTING CONDITIONS BEFORE SUBMITTING BID. ANY DISCREPANCIES BETWEEN THESE DRAWINGS AND THE EXISTING CONDITIONS SHALL BE BROUGHT TO THE ATTENTION TO THE OWNER AND ARCHITECT BY THE G.C. PRIOR TO THE START OF ANY WORK.
- 2. G.C. SHALL COORDINATE ALL WORK TO BE PERFORMED. G.C. IS TO COORDINATE WORKING HOURS, DELIVERIES, TRASH REMOVAL, STORAGE, ETC. WITH THE OWNER.
- G.C. SHALL REMOVE ALL DEBRIS AND LEAVE JOB SITE CLEAN 3. UPON COMPLETION OF WORK.
- 4. G.C. SHALL BEAR FULL RESPONSIBILITY AND COSTS FOR THE FOLLOWING: - ALL PERMITS, LICENSES, AND FEES.
- PRIOR TO BEGINNING WORK THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITIES FROM DAMAGE DURING CONSTRUCTION. SHOULD DAMAGE OCCUR TO ANY UTILITIES AS A RESULT OF CONSTRUCTION ACTIVITIES, THE CONTRACTOR SHALL BEAR RESPONSIBILITY FOR REPAIRS AND ALL COSTS.
- CONSULT WITH OWNER FOR ALL EQUIPMENT, FIXTURE, & FINISH 6. SELECTIONS.
- ELECTRICAL CONTRACTOR TO PROVIDE ALL REQUIRED INFORMATION TO JURISDICTION FOR ALL ELECTRICAL SYSTEMS, PANELS, AND EQUIPMENT INCLUDING SIZE OF SERVICE AND LOCATION OF EQUIPMENT.
- MECHANICAL CONTRACTOR TO PROVIDE ALL REQUIRED INFORMATION TO JURISDICTION FOR ALL MECHANICAL SYSTEMS INCLUDING SIZE OF DUCTS, TYPE AND SIZE OF MECHANICAL EQUIPMENT AND LOCATION OF ALL EQUIPMENT.
- PLUMBING CONTRACTOR TO PROVIDE ALL REQUIRED INFORMATION TO JURISDICTION FOR ALL PLUMBING SYSTEMS INCLUDING TYPE, SIZE, AND LOCATION OF COMPONENTS ASSOCIATED WITH PLUMBING SYSTEMS
- PROJECT DESCRIPTION SINGLE FAMILY DWELLING - RENOVATION JURISDICTION CITY OF DECATUR APPLICABLE CODES INCLUDE AND NOT LIMITED TO: 2018 INTERNATIONAL BUILDING CODE W/ 2020 GA AMENDMENTS 2018 INTERNATIONAL RESIDENTIAL CODE FOR ONE & TWO-FAMILY DWELLINGS W/ 2020 GA AMENDMENTS 2018 INTERNATIONAL PLUMBING CODE W/ 2020 GAAMENDMENTS 2018 INTERNATIONAL MECHANICAL CODE W/ 2020 GA AMENDMENTS 2018 INTERNATIONAL FUEL GAS CODE W/ 2020 GA AMENDMENTS 2015 INTERNATIONAL ENERGY CONSERVATION CODE W/ 2020 GA AMENDMENTS 2018 INTERNATIONAL EXISTING BUILDING CODE W/ 2015 GA AMENDMENTS 2018 INTERNATIONAL FIRE CODE W/ GA AMENDMENTS 2020 NFPA NATIONAL ELECTRICAL CODE W/ NO GA AMENDMENTS 2018 NFPA LIFE SAFETY CODE W/ 2020 GA AMENDMENTS GEORGIA HANDICAPPED ACCESSIBILITY LAW 120-3-20 FLOOR AREA CALCULATION: HOUSE SECOND 640 FIRST 1783 SUB-TOTAL 2423 sq ft GARAGE 243 TOTAL 2666 sq ft **BUILDING HEIGHT:** TWO STORY: 29'-1" **OCCUPANCY CLASSIFICATION:** DESIGN LOADS:

FLOOR: LIVE 20psf DEAD 40psf

ROOF: LIVE 20psf DEAD 20psf

SINGLE FAMILY RESIDENCE OCCUPANT LOAD: N/A

CONSTRUCTION TYPE: TYPE VB

**CITY OF DECATUR** 



ARCHITECTURAL							
A-1.1	Site Plan						
A-2.1	Demolition Plan						
A-2.2	Floor Plans						
A-2.3	Floor Plans						
A-3.1	Exterior Elevations						
A-3.2	Exterior Elevations						
A-4.1	Building Sections						

IS DRAWING IS THE EXC PROPERTY OF THE ARCHITED AND MAY NOT BE REPRODUC OR USED IN ANY WAY WITHO AUTHORIZATION AND WRITTE FRMISSION COPYRIGHT © 2 BY RAWLINGS DESIGN, INC Revision Drawing Date: 1/17/24 **RELEASED FOR** CONSTRUCTION **Project Number:** 2320 ш NCI Ш SIDI Ш R FRIEDENTHAL-GREENE **Drawing Description:** 





#### DOOR SCHEDULE

			DOOR			FRAME			DEMARKS		
NO.			TYPE	LABEL	MATERIAL	TYPE	HEAD	JAMB	SILL	MATERIAL	REMARKS
106.1	3'-0"	6'-8"									
106.2	2'-6"	6'-8"									
201.1	3'-0"	6'-8"									
202.1	2'-6"	6'-8"									
202.2	2'-0"	6'-8"									
203.1	2'-4"	6'-8"									
204.1	2'-4"	6'-8"									
205.1	2'-4"	6'-8"									
206.1	2'-6"	6'-8"									



NO. WIDTH				неюнт								ТУРГ				DETAILS				DEMARKO
	WIDTH		ELEV	TTPE	MATERIAL	LINIQU	GLAZING	HEAD	JAMB	SILL	MULLION	REMARKS								
108A	4'-0"	2'-0"																		
201A	8'-0"	3'-0"					TEMPERED													
201B	4'-0"	2'-0"					TEMPERED													
202A	5'-4"	5'-2"										EGRESS								
202B	4'-0"	2'-0"					TEMPERED													
203A	3'-0"	2'-0"					TEMPERED													
204A	3'-0"	2'-0"					TEMPERED													
206A	4'-0"	2'-0"																		
206B	5'-4"	5'-2"										EGRESS								

### WINDOW SCHEDULE

























	NEW ASPHALT SHINGLES				
NEW SECOND LEVI	NEW 5/4X6 TRIM BAND NEW WOOD WINDOWS NEW 1X4 CORNER TRIM, TYP.				
2 SE	NEW CEMENT BOARD LAP SIDING +9'-11"		 · · · _	 	
EXISTING FIRST LEVEL					]  
+ $ 1$			 	 	



#### DESIGN CRITERIA NOTES

- I. GOVERNING BUILDING CODE: 2018 INTERNATIONAL RESIDENTIAL CODE w/ GA AMENDMENTS. ANY STANDARD REFERENCED IN THESE PLANS IS TO BE THE EDITION SPECIFIED IN THE REFERENCED STANDARDS CHAPTER OF THE GOVERNING BUILDING CODE. ANY STANDARD NOT LISTED IN THE REFERENCED STANDARDS CHAPTER IS TO BE THE LATEST EDITION
- 2. WIND LOADS V<sub>ult</sub> = 115 MPH 2.1 BASIC WIND SPEED -- 3 SECOND GUST 2.2 IMPORTANCE FACTOR 2.3 WIND EXPOSURE CATEGORY 2.4 DESIGN WIND PRESSURE FOR COMPONENTS & CLADDING \* 2.4.1 ROOF (ASD) -36.1/+13.1 PSF 2.4.2 WALL (ASD) -19.1/+14.3 PSF 5 PSF
- 3. SNOW GROUND LOAD, pg
- 4. REFER TO PLANS FOR DEAD *≰* LIVE LOADS AT EACH LEVEL

\* DESIGN WIND PRESSURES FOR COMPONENTS & CLADDING REPRESENT WORST CASE OUTER ZONES. DESIGN PRESSURES USED BY MANUFACTURERS MAY BE REDUCED PROVIDED THE COMPONENT OR CLADDING IS LOCATED WITHIN AN INNER ZONE DESIGNED TO RESIST THE CORRESPONDING INNER ZONE PRESSURE W/ APPLICABLE ADJUSTMENT FACTORS LISTED IN THE GOVERNING BUILDING CODE OR ASCE-7.

#### GENERAL STRUCTURAL NOTES

- I. THE ARCHITECTURAL LAYOUT AND BUILDING PLACEMENT ON THE LOT IS NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER OF RECORD OR ANY KOBLASZ & KENNISON ENGINEERING PERSONNEL, HENCEFORTH REFERRED TO AS "THE ENGINEER". THEREFORE, THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS W/ THE DRAWINGS OF OTHER DISCIPLINES THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE PRIOR TO CONSTRUCTION. DO NOT SCALE DRAWINGS TO DETERMINE DIMENSIONS.
- 2. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS AND SPECIFICATIONS OF ALL OTHER DISCIPLINES. PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES, SUCH AS, BUT NOT LIMITED TO, SLEEVES, CHASES, RECESSED LIGHTING, HANGERS, INSERTS, ANCHORS HOLES AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK. THE CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY IF ANY CONFLICTS ARE DISCOVERED.
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE ENGINEER WILL NOT ADVISE ON NOR ISSUE DIRECTION AS TO SAFETY PRECAUTIONS AND
- 4. THE STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL ALL STRUCTURAL WORK AND CONNECTIONS HAVE BEEN COMPLETED. THE INVESTIGATION, DESIGN, SAFETY, ADEQUACY AND INSPECTION OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR
- 5. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE METHODS, TECHNIQUES, AND SEQUENCES OF THE PROCEDURES TO PERFORM THE WORK. THE SUPERVISION OF THE WORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 6. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO APPROVAL BY THE ENGINEER.
- 7. ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH THE SUPPLIER'S INSTRUCTIONS AND REQUIREMENTS.
- 8. LOADING APPLIED TO THE STRUCTURE DURING THE PROCESS OF CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS INDICATED ON THE PLANS. DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY CONNECTED TOGETHER AND UNTIL ALL TEMPORARY BRACING IS IN PLACE
- 9. IN ACCORDANCE WITH THE APPLICABLE CODE, SPECIAL INSPECTIONS MAY BE REQUIRED. THE OWNER OR CONTRACTOR SHALL HIRE THE SPECIAL INSPECTOR TO PERFORM ALL REQUIRED SPECIAL INSPECTIONS.
- 10. SHOP DRAWINGS AND OTHER ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR SUBMITTING TO THE ENGINEER. THE ENGINEER'S REVIEW IS TO BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE RELEVANT CONTRACT DOCUMENTS. THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW, CHECK AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSIONS. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR THE ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC. THE ENGINEER SHALL BE PROVIDED SHOP DRAWINGS ALLOWING NO LESS THAN (2) WEEKS FROM RECEIPT FOR THE ENGINEER TO COMPLETE EACH SHOP DRAWING REVIEW.
- II. ALL DELEGATED DESIGN OR DESIGN BY OTHERS IS TO BE DESIGNED AND SPECIFIED BY A REGISTERED ENGINEER OR ARCHITECT, WHICHEVER IS APPLICABLE TO THE DESIGN, IN THE STATE THE PROJECT IS TO BE BUILT AND, IF INCLUDED IN THE CONTRACT, SUBMITTED FOR APPROVAL. THE ENGINEER CLAIMS NO RESPONSIBILITY FOR DELEGATED DESIGN OR DESIGN BY OTHERS IF THE DESIGN AND CALCULATIONS, IF APPLICABLE, WERE NOT SUBMITTED TO THE ENGINEER
- 1.2. THE ENGINEER IS NOT RESPONSIBLE FOR DESIGN TO PREVENT WATER INTRUSION. THE CONTRACTOR SHALL PROVIDE AND FOLLOW THE DRAWINGS AND SPECIFICATIONS OF OTHER TRADES AS IT PERTAINS TO WATERPROOFING.
- 1.3. THE DESIGN HEREIN BELONG TO THE ENGINEER. A LICENSE TO CONSTRUCT THIS BUILDING FROM THESE PLANS AT A SINGLE SITE IS GRANTED TO THE CONTRACTED CLIENT. LICENSE IS NON-TRANSFERABLE. ANY BREACH OF THIS LICENSE SHALL ENTITLE THE ENGINEER TO PURSUE ANY AND ALL REMEDIES, AT LAW OR EQUITY, INCLUDING WITHOUT LIMITATION, INJUNCTIVE RELIEF TO PREVENT OR CEASE SUCH BREACH.
- 14. IT IS THE RESPONSIBILITY OF PURCHASER OF PLANS TO ENSURE THE FOLLOWING BEFORE CONSTRUCTION: 14.1. CONTRACTOR MUST VERIFY ALL DIMENSIONS PRIOR TO PROCEEDING WITH CONSTRUCTION. 14.2. CONTRACTOR MUST VERIFY COMPLIANCE WITH ALL LOCAL BUILDING CODES IN THE AREA THE PROJECT IS TO BE BUILT 14.3. ENGINEERING CONSULTANTS MUST INCORPORATE ACTUAL SITE CONDITIONS.
- 14.4. ANY MODIFICATIONS TO THESE DOCUMENTS MUST BE MADE BY THE ENGINEER. 14.5. PLANS INDICATE LOCATION ONLY. SITE CONDITIONS MUST BE VERIFIED BY OTHERS AND ACTUAL SITE

CONDITION MUST BE INCORPORATED INTO ENGINEERING ASPECTS.

- FOUNDATION NOTES
- I. ALL FOOTINGS SHALL BEAR ON UNDISTURBED, FIRM, NATURAL SOIL OR ENGINEERED SOIL CAPABLE OF SUPPORTING A MINIMUM DESIGN BEARING PRESSURE OF 2,000 PSF.
- 2. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY A GEOTECHNICAL ENGINEER / TESTING AGENCY PRIOR TO POURING FOUNDATION CONCRETE. THE CONTRACTOR SHALL ENGAGE THE GEOTECHNICAL ENGINEER. FOOTING INSPECTIONS BY KOBLASZ & KENNISON ENGINEERING SHALL NOT REPLACE THE NEED FOR A GEOTECHNICAL ENGINEER.
- 3. CONTRACTOR TO PROVIDE TEMPORARY SHORING TO BRACE FOUNDATION WALLS PRIOR TO BACK FILLING OR FLOOR SYSTEM SHALL BE INSTALLED PRIOR TO BACK FILLING TO BRACE TOP OF WALL, UNLESS NOTED OTHERWISE (U.N.O.).
- 4. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTING UTILITIES. FOUNDATIONS SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES AND COORDINATED W/ THE ENGINEER.
- 5. PROVIDE PROPER AND ADEQUATE DRAINAGE BEHIND ANY TYPE OF RETAINING AND/OR BASEMENT WALLS AS THE SITE CONDITIONS REQUIRE IN THE FIELD.
- 6. ALL BOTTOM OF ALL FOOTINGS AND FOUNDATIONS SHALL BE PLACED BELOW THE FROST DEPTH OF THE GEOGRAPHIC AREA OF THE PROJECT.
- 7. IN THE PRESENCE OF THE GROUND WATER TABLE ABOVE ANY FOOTING OR FOUNDATION, THE CONTRACTOR SHALL NOTIFY THE ENGINEER FOR POTENTIAL DESIGN REVISIONS.
- 8. ALL STEEL EXPOSED TO WATER, MOISTURE, AND/OR CORROSIVES SHALL BE COVERED WITH APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS.
- 9. THE FOLLOWING PROPERTIES ARE USED IN THE DESIGN OF WALLS, SLABS, AND ANY OTHER MEMBERS RETAINING SOIL AND MUST BE VERIFIED BY A GEOTECHNICAL ENGINEER: 9.1 SOIL SPECIFIC WEIGHT (SOIL DENSITY COMPACTED TO 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY BASED ON ASTM D698 METHOD),  $\gamma$ = 110 PCF
- 9.2 INTERNAL FRICTION ANGLE,  $\Phi$ = 30°
- 9.2 EXTERNAL FRICTION FACTOR,  $\mu = 0.35$ 9.3 ACTIVE SOIL PRESSURE (or EFP), Pa= 36.7 PSF
- 9.4 PASSIVE SOIL PRESSURE, Pp= 330 PSF

CAST-IN-PLACE CONCRETE NOTES

I. CONCRETE MIXES AND CONCRETE WORK SHALL CONFORM TO ACI 30 I USING PORTLAND CEMENT, AGGREGATES AND ADMIXTURES CONFORMING TO ASTM REQUIREMENTS. CONCRETE SHALL BE READY-MIXED IN ACCORDANCE WITH ASTM REQUIREMENTS

2.	CONCRETE SHALL CONFORM TO THE FOLLOWING COMPRESSIVE STRENGTH AND WATER/CEMENT RATIO REQU						
	CONCRETE	MIN. fc (28 DAYS)	MAX. W/C RATIO				
	COLUMNS	4,000 PSI	0.46				
	ELEVATED SLABS	4,000 PSI	0.46				
	CONCRETE NOT NOTED	3,000 PSI	0.50				
	FOUNDATION	3,000 PSI	0.50				
	SLABS-ON-GRADE	3,000 PSI	0.50				

- 3. ALL REINFORCING STEEL SHALL CONFORM TO ASTM REQUIREMENTS GRADE 60. ALL WELDII SHALL BE IN ACCORDANCE WITH AWS REQUIREMENTS AND ALL WELDED REBAR SHALL BE WELDABLE REBAR. EPOXY COATED REINFORCING SHALL CONFORM TO ASTM REQUIREMENTS.
- 4. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM REQUIREMENTS.
- 5. ALL REINFORCING STEEL SHALL BE SET AND TIED IN PLACE PRIOR TO POURING OF CONCRETE. DO NOT FIELD BEND BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE, U.N.O.
- 6. REINFORCING STEEL, INCLUDING HOOKS AND BENDS, SHALL BE DETAILED IN ACCORDANCE WITH ACI 3 1 5. ALL REINFORCING STEEL LAP SPLICES SHALL BE CLASS B, U.N.O.
- 7. MINIMUM CONCRETE COVER SHALL BE PROVIDED IN ACCORDANCE WITH ACI 318.
- 8. BAR SUPPORTS SHALL BE PROVIDED FOR ALL REINFORCING STEEL TO INSURE MINIMUM SUPPORT AND HOLDING BARS PER CONCRETE COVER. BAR SUPPORTS SHALL BE PLASTIC TIPPED OR STAINLESS STEEL.
- 9. ALL EDGES OF PERMANENTLY EXPOSED CONCRETE SURFACES SHALL BE CHAMFERED, U.N.O. 10. FORM WORK SHALL REMAIN IN PLACE UNTIL CONCRETE HAS OBTAINED AT LEAST 90% OF ITS 28 DAY COMPRESSIVE STRENGTH. THE CONTRACTOR SHALL PROVIDE ALL SHORING AND RE-SHORING.
- II. PROVIDE JOINTS IN ALL CONCRETE WORK AS REQUIRED BY THE ACI CODE, U.N.O.
- 12. THE MAXIMUM LENGTH OF SLAB CAST IN ANY ONE CONTINUOUS POUR SHALL BE LIMITED TO 80 FEET, U.N.O.

#### WOOD FRAMING NOTES

- I. ALL 2x4, 2x6, 2x8 WOOD FRAMING SHALL BE SPF #2. ALL 2x10, 2x12 FRAMING SHALL BE SYP #2, U.N.O.
- 2. ALL 4x, 6x, 8x, 10x, 12x WOOD FRAMING SHALL BE SYP #2 OR DF #2, U.N.O.
- 3. ALL INTERIOR FRAMING SHALL BE SURFACE DRIED OR KILN DRIED AND INSTALLED AT 19% MAXIMUM MOISTURE CONTENT.
- 4. ALL FRAMING EXPOSED TO THE WEATHER OR IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED IN ACCORDANCE WITH THE AMERICAN WOOD PRESERVER'S ASSOCIATION SPECIFICATIONS, UNLESS NOTED OTHERWISE (U.N.O.). WHERE POSSIBLE, ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO ON-SITE FABRICATION SHALL BE BRUSHED WITH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STD. M4).
- 5. ALL PRE-FABRICATED STRUCTURAL COMPOSITE LUMBER SHALL BE MANUFACTURED BY BOISE CASCADE OR AN APPROVED EQUAL. DRILLED HOLES SHALL BE IN ACCORDANCE W/ MANUFACTURER'S WRITTEN REQUIREMENTS, U.N.O. 5.1 ALL LVL (LAMINATED VENEER LUMBER) SHALL BE  $|\frac{2}{3}^{u}$  PLY THICKNESS, E  $\geq 2.0 \times 10^{6}$  PSI, F<sub>b</sub>  $\geq 2,650$  PSI, F<sub>v</sub>  $\geq$ 285 PSI.
- 6. ALL NAILING SHALL BE IN ACCORDANCE WITH THE FASTENER SCHEDULE ON THIS SHEET, U.N.O., AND NAILING SHALL NOT BE OVERDRIVEN.
- 7. BOLTS AND THREADED RODS USED TO SECURE WOOD MEMBERS SHALL BE ASTM A307 GRADE, U.N.O.
- 8. BOLT AND THREADED ROD HOLES SHALL BE CAREFULLY CENTERED AND DRILLED NOT MORE THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTED CONNECTIONS SHALL BE SNUG - TIGHT BUT NOT TO THE EXTENT OF CRUSHING WOOD UNDER WASHERS
- 9. LAG SCREWS AND WOOD SCREWS ARE TO HAVE LEAD HOLES PRE-DRILLED IN ACCORDANCE W/ THE NDS FOR WOOD CONSTRUCTION. THE REQUIRED LEAD HOLE DIAMETER DEPENDS ON THE TYPE OF LOADING, TYPE OF SCREW, AND SPECIFIC GRAVITY OF THE WOOD.
- 10. ALL BOLTS, THREADED RODS, AND LAG SCREWS WITH A NUT OR HEAD IN CONTACT WITH WOOD REQUIRE A FLAT WASHER. WASHERS ARE TO HAVE AN INSIDE DIAMETER, OUTSIDE DIAMETER, AND THICKNESS IN ACCORDANCE W/ USS WASHER SPECIFICATIONS, U.N.O.
- II. ALL BOLTS, THREADED RODS, AND LAG SCREWS GREATER THAN  $\frac{1}{4}$ " DIAMETER ARE TO HAVE AN O.C. SPACING, WOOD EDGE DISTANCE, AND WOOD END DISTANCE NOT LESS THAN 4x FASTENER DIAMETER, U.N.O.
- PREFABRICATED METAL JOIST HANGERS, HURRICANE CLIPS, HOLD-DOWN ANCHORS, POST BASES/CAPS, AND OTHER. ACCESSORIES SHALL BE MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY", OR AN APPROVED EQUAL. INSTALL ALL ACCESSORIES AND FASTENERS PER THE MANUFACTURER'S REQUIREMENTS WITH "MAX" NUMBER OF MANUFACTURER SPECIFIED FASTENERS. NAILS ARE TO BE FULL LENGTH (10d COMMON = 3" LONG, 16d COMMON =  $3\frac{1}{2}$ " LONG).
- 13. ALL HARDWARE AND FASTENERS USED FOR PRESSURE TREATED WOOD SHALL BE MADE FROM APPROVED CORROSIVE-RESISTANT MATERIALS.
- 14. THE ENDS OF ALL BEAMS \$ JOISTS ARE TO BE RESTRAINED TO PREVENT ROTATION. ALL FLUSH BEAMS ARE TO BE CONTINUOUSLY BRACED ALONG THE SIDES. ALL DROPPED BEAMS ARE TO BE CONTINUOUSLY BRACED ALONG THE TOP FACE
- 15. LAP FLOOR & CEILING JOISTS BY THE THICKNESS OF BEARING WALL (MIN. 3") & DO NOT EXTEND BEYOND THE WALL, U.N.O
- I.G. IN FLOOR CAVITIES PROVIDE BLOCKING UNDER ALL CONCENTRATED LOADS, INCLUDING SUPPORTS FOR ALL BEAMS # HEADERS, AND INSTALL CRIPPLE STUDS / PONY WALL UNDER ALL STACKING LOAD BEARING WALLS.
- 17. ALL STUDS TO BE CONTINUOUS BETWEEN DIAPHRAGMS.
- 18. ALL COLUMNS TO BE BRACED AT TOP \$ BOTTOM. ALL CONTINUOUS COLUMNS TO BE BRACED AT EACH FLOOR LEVEL.

#### PREFABRICATED WOOD JOIST NOTES

- I. PREFABRICATED WOOD I-JOISTS SHALL BE DESIGNED AND FURNISHED IN ACCORDANCE WITH A CURRENT CODE-ACCEPTED EVALUATION REPORT. STRUCTURAL CAPACITIES AND DESIGN PROVISIONS SHALL BE ESTABLISHED AND MONITORED IN ACCORDANCE WITH ASTM D-5055.
- 2. WOOD I-JOIST SHALL NOT BE CUT, NOTCHED, COPED, DRILLED, NOR OTHERWISE ALTERED IN ANY WAY UNLESS SPECIFICALLY CONDUCTED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN REQUIREMENTS. DO NOT DRILL THROUGH FLANGES.
- 3. WOOD I-JOISTS SHALL BE PRODUCED BY A CODE ACCEPTED FABRICATOR WITH A MINIMUM OF FIVE (5) YEARS EXPERIENCE PRODUCING PREFABRICATED WOOD I-JOISTS. QUALITY CONTROL SHALL BE AUDITED BY AN AGENCY ACCEPTED BY THE "BUILDING OFFICIALS & CODE ADMINISTRATORS, INC."
- 4. WOOD I-JOISTS SHALL BE STORED IN BUNDLES IN AN UPRIGHT POSITION AND AWAY FROM GROUND CONTACT. DAMAGE TO JOISTS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE JOIST SUPPLIER. FIELD REPAIR OR MODIFICATION OF JOISTS MUST NOT BE MADE WITHOUT THE WRITTEN APPROVAL BY THE SUPPLIER, EXCEPT FOR TRIMMING TO CORRECT LENGTH.
- 5. WOOD I-JOISTS SHALL BE CAREFULLY HANDLED TO PREVENT DAMAGE AND DISTORTION. EACH JOIST SHALL BE ANCHORED AND BRACED AS IT IS ERECTED USING BLOCKING PANELS AND ANCHORAGE INDICATED (AND PER THE SUPPLIERS REQUIREMENTS). ERECTOR SHALL PROVIDE SUPPLEMENTAL LATERAL BRACING OF THE TOP FLANGE UNTIL SHEATHING IS PROPERLY NAILED.

JOIST DIRECTION LEGEND

![](_page_13_Figure_73.jpeg)

STRUCTURAL STEEL NOTES

HSS RECT.

PIPE

PLATES

HSS ROUND

I. ALL STRUCTURAL STEEL SHALL CONFORM TO THE "STEEL CONSTRUCTION MANUAL" OF THE AISC.

A36

A500 GR. B

A500 GR. B

A53 GR. B

	2. UNLESS NOTED OTHERW	/ISE, ALL MATERIALS SHALL	BE IN ACCORDANCE WITH THE FOLLOWING SF	'ECIFICATIONS:
UIREMENTS:	MEMBER W-SHAPFS	ASTM A992	$\frac{\text{MIN. STRENGTH}}{\text{Ev} = 50 \text{ KSL}} = 65 \text{ KSL}$	

M, S, C, MC, L-SHAPES

CONNECTION BOLTS

ANCHOR BOLTS

THREADED RODS

NON-SHRINK GROUT

STRENGTH OF 70 KSI

ING OF	REINFORCING STEEL	

 $F_V = 36 \text{ KSI}$ .  $F_U = 58 \text{ KSI}$ A36 A325  $F_{U} = 120 \text{ KSI}$ F1554 GR. 36 Fy = 36 KSI, Fu = 58 KSI A36  $F_y = 36 \text{ KSI}, F_U = 58 \text{ KSI}$ CI107 5,000 PSI 3. ALL WELDING SHALL BE IN ACCORDANCE WITH AWS DI. I USING E70XX ELECTRODES, U.N.O. PROVIDE CONTINUOUS MINIMUM SIZED FILLET WELDS PER AWS REQUIREMENTS. ALL FILLER MATERIAL SHALL HAVE A MINIMUM TENSILE 4. ALL STRUCTURAL STEEL IN SSPC ENVIRONMENTAL ZONES 0 AND 1 A SHALL BE SHOP PAINTED WITH ONE COAT OF SSPC 15-68. TYPE 1 (RED OXIDE) PAINT, U.N.O. 5. ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WITH APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS. 6. PROTECTIVE COATINGS DAMAGED DURING THE TRANSPORTING, ERECTING, AND FIELD WELDING PROCESSES SHALL BE REPAIRED IN THE FIELD TO MATCH THE SHOP APPLIED COATING. 7. ALL BEAM COPES SHALL BE MADE W/ MINIMAL COPE DIMENSIONS - 2" CLEAR DIMENSION FOR NON-CONNECTED SURFACES IS PERMISSIBLE. ALL RE-ENTRANT CORNERS SHALL HAVE A MINIMUM  $\frac{1}{2}$ " RADIUS. 8. ALL BOLTS SHALL BE SHEAR/BEARING TYPE BOLTS AND BE "SNUG-TIGHT", U.N.O. 9. FOR ALL BOLTS  $\frac{7}{6}$  DIAMETER AND LESS, HOLES SHALL BE STANDARD HOLES  $\frac{1}{16}$  LARGER THAN BOLT DIAMETER. ALL BOLT HOLES SHALL BE DRILLED, PUNCHED, OR THERMALLY CUT BY MECHANICALLY GUIDED EQUIPMENT. THERMALLY CUT HOLES SHALL HAVE A SURFACE ROUGHNESS NOT EXCEEDING 1,000 MICROINCHES. IO. WASHERS, WHERE REQUIRED, SHALL BE ASTM F436, U.N.O. AND SHALL MATCH BOLT FINISH. FOR SNUG-TIGHT JOINTS, WASHERS ARE ONLY REQUIRED IN THE FOLLOWING CONDITIONS: WHEN THE OUTER FACE OF THE JOINT HAS A SLOPE THAT IS GREATER THAN 1:20 W/ RESPECT TO A PLANE NORMAL TO BOLT AXIS, AN ASTM F436 BEVELED WASHER SHALL BE USED; WHEN A SLOTTED HOLE OCCURS IN THE OUTER PLY, AN ASTM F436 OR 12: COMMON PLATE WASHER SHALL BE USED AS REQUIRED TO COMPLETELY COVER THE HOLE. II. WHERE STEEL BEAMS ARE FLUSH AND SUPPORT JOISTS ON HANGERS, PACK OUT THE WEB W/ SYP OR LVL NAILER PLATES CUT TO FIT TIGHT BETWEEN FLANGES AND SECURE W/ 5" DIAMETER A307 THRU BOLTS @ 16" O.C. STAGGERED. THE EDGE OF THE NAILER PLATES ARE TO BE FLUSH w/ THE EDGE OF THE FLANGE.

 $y = 50 \text{ KSI}, F_{U} = 65 \text{ KSI}$ 

Fy = 36 KSI, Fu = 58 KSI

Fy = 46 KSI, Fu = 58 KSI

 $F_{V} = 42 \text{ KSI}, F_{U} = 58 \text{ KSI}$ 

Fy = 35 KSI, Fu = 60 KSI

MASONRY NOTES

- I. MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF ACI 530.
- 2. HOLLOW LOAD-BEARING MASONRY UNITS SHALL CONFORM TO ASTM REQUIREMENTS. THE MINIMUM PRISM COMPRESSIVE STRENGTH (f 'm) SHALL BE 1,500 PSI AT AN AGE OF 28 DAYS, AS DETERMINED BY THE UNIT STRENGTH METHOD OF ACI 530.
- 3. FILL ALL BOND BEAMS AND REINFORCED CELLS SOLIDLY WITH GROUT. GROUT SHALL CONFORM TO ASTM REQUIREMENTS AND SHALL OBTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2,500 PSI.
- 4. THE USE OF MASONRY-CEMENT MORTAR IS STRICTLY PROHIBITED. MORTAR SHALL CONFORM TO ASTM REQUIREMENTS: ALL MORTAR SHALL MEET THE "PROPORTION SPECIFICATION" OF ASTM REQUIREMENTS AND BE MADE WITH PORTLAND CEMENT LIME (NON AIR-ENTRAINED).
- 5. UNLESS NOTED OTHERWISE, ALL WALLS SHALL BE LAID IN RUNNING BOND. BOND CORNERS AND INTERSECTIONS OF ALL WALLS.
- 6. REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM REQUIREMENTS, GRADE 60. PROVIDE A MINIMUM LAP OF 48 TIMES THE BAR DIAMETERS AT ALL SPLICES, U.N.O.
- 7. VERTICAL REINFORCEMENT TO BE MINIMUM (1) #4 BAR AT CORNERS, ON EACH SIDE OF OPENINGS, AT THE ENDS OF WALLS, AND AT INTERSECTIONS w/ A MAXIMUM SPACING OF 4'-0" O.C. BETWEEN.
- 8. PROVIDE REBAR DOWELS FROM FOUNDATIONS TO MATCH VERTICAL REINFORCING SIZE AND SPACING. DOWELS SHALL HAVE STANDARD 90 DEGREE HOOKS AND LAP WITH THE FIRST LIFT OF REINFORCING OR PER LAP NOTE ABOVE, WHICHEVER IS GREATER.
- 9. MINIMUM GROUT \$ MASONRY COVER SHALL BE PROVIDED IN ACCORDANCE WITH ACI 530.
- PROVIDE BOND BEAM UNTELS ABOVE ALL WALL OPENINGS PER THE SPECIFICATIONS ON THE PLANS W/ NO LESS THAN 4" OF BEARING, U.N.O. THE ENTIRE LINTEL DEPTH SHALL BE COMPOSED OF U-SHAPED BOND BEAM COURSES AND FULLY GROUTED.
- 1. PROVIDE STEEL JOIST AND BEAM BEARING PLATES AND OTHER ACCESSORIES AS INDICATED ON THE PLANS. PROVIDE SOLIDLY GROUTED CMU BELOW ALL BEAM BEARINGS OVER THE WIDTH OF THE BEARING PLATE AND ONE CELL BEYOND ON EACH SIDE.
- 12. PROVIDE JOINTS IN ALL MASONRY WORK AS REQUIRED BY THE ACI CODE OR NCMA TEK DOCUMENTS, U.N.O.

#### ANCHORED VENEER NOTES

- 1. THE DESIGN OF VENEER IS NOT INCLUDED IN THE SCOPE OF THESE PLANS. PLANS INCLUDE DESIGN OF ELEMENTS SUPPORTING VENEER, BUT NOT THE VENEER ITSELF, WHICH INCLUDES MASONRY/MORTAR SPECIFICATIONS AND MOVEMENT JOINTS. MOVEMENT JOINTS ARE TO BE SPECIFIED BY OTHERS AND ARE TO CONFORM TO THE APPLICABLE BUILDING CODE.
- 2. VENEER WITH A 1" NOMINAL AIR SPACE SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS W/ CORROSION-RESISTANT 22 U.S. GAGE METAL TIES w/ 8d COMMON NAILS AT 16" O.C. HORZ. \$ 24" O.C VERT. METAL TIES SHALL BE EMBEDDED IN MORTAR OR GROUT AND EXTEND INTO THE VENEER A MIN. OF 1/3", w/ NOT LESS THAN 🐉 MORTAR OR GROUT COVER TO OUTSIDE FACE. REFER TO THE APPLICABLE BUILDING CODE FOR ADDITIONAL ANCHOR REQUIREMENTS ADJACENT TO OPENINGS AND IF AIR SPACE IS GREATER THAN I".
- 3. LINTELS SPANNING OVER OPENINGS SHALL BE IN ACCORDANCE w/ THE TABLES BELOW, U.N.O. LINTELS ARE NOT TO BE ATTACHED TO THE FRAMING, U.N.O. IF ANY SPANS EXCEED THE TABLES BELOW AND ARE NOT SPECIFIED ON THE PLANS, CONTRACTOR IS TO CONTACT THE ENGINEER IMMEDIATELY. LINTELS ARE TO HAVE 4" OF BEARING ON THE MASONRY ON EACH END (THIS APPLIES TO LINTELS THAT ARE FREE-SPANNING AND LINTELS THAT ARE ATTACHED TO FRAMING).

#### ALLOWABLE CLEAR SPANS FOR LINTELS SUPPORTING 4" NOMINAL CLAY BRICK VENEER WEIGHING < 38 PSF

SIZE OF STEEL ANCLE	HEIGHT OF VENEER ABOVE LINTEL							
SIZE OF STEEL ANGEL	2'-0"	4'-0"	6'-0"	8'-0"	10'-0"	15'-0"	20'-0"	25'-0"
L $3\frac{1}{2}x \ 3\frac{1}{2}x \ \frac{1}{4}$ "	8'-4"	6'-7"	5'-8"	5'-2"	4'-9"	4'-0"	3'-7"	3'-4"
L 6x 3½x $\frac{3}{8}$ a	16'-0"	2'-6"	0'-  "	9'-10"	9'-2"	8'-0"	7'-3"	6'-8"
(2) L Gx 3 <sup>1</sup> / <sub>2</sub> x <sup>3</sup> / <sub>8</sub>	20'-2"	16'-0"	4'-  "	12'-6"	'-8"	10'-2"	9'-2"	8'-5"

<sup>a.</sup> LONG LEG OF THE ANGLE SHALL BE PLACED IN A VERTICAL POSITION <sup>b.</sup> INSTALL FIRST LINTEL, ADD COURSES UNTIL VENEER CLEARS TOP OF FIRST LINTEL, THEN ADD SECOND LINTEL (LINTELS ARE STACKED ATOP EACH OTHER AND NOT CONNECTED TO EACH OTHER)

#### ALLOWABLE CLEAR SPANS FOR LINTELS SUPPORTING 6" MAX STONE VENEER WEIGHING < 82 PSF HEIGHT OF VENEER ABOVE LINTEL SIZE OF STEEL ANGL

	2'-0"	4'-0"	6'-0"	8'-0"	10'-0"	l 5'-0"	20'-0"	25'-0"
L 5x 5x <del>3</del> 8"	10'-9"	8'-5"	7'-4"	6'-8"	6'-2"	5'-3"	4'-9"	4'-4"
L 6x 6x <sup>3</sup> "	3'-0"	10'-3"	8'-10"	8'-0"	7'-5"	6'-5"	5'-10"	5'-4"
L 8x 6x <del>7</del> " ª	4'-2"	4'-2"	2'-4"	'-2"	10'-4"	9'-0"	8'-2"	7'-6"

<sup>a.</sup> LONG LEG OF THE ANGLE SHALL BE PLACED IN A VERTICAL POSITION

IF ANY CONNECTIONS DESCRIBED IN T DESCRIPTION OF BUILD DESCRIPTION OF BUILD CEILING JOIST TO TOP PLATE CEILING JOIST TO TOP PLATE CEILING JOIST TO CEILING JOIST (LA S. RAFTER OR ROOF TRUSS TO TOP PL G. RAFTER TO RIDGE, VALLEY, OR HIP P C. RAFTER TO RIDGE, VALLEY, OR HIP C. ROMINUOUS HEADER TO STUDS S. ABUTTING STUD S AT INTERSECTING CONTINUOUS HEADER TO STUD CONTINUOUS HEADER TO STUD C. NING STUD TO END OF HEADER C. DOUBLE TOP PLATE SPLICE (24" MIN C. SING STUD TO END OF HEADER C. DOUBLE TOP PLATE SPLICE (24" MIN C. DOUBLE TOP PLATE TO JOIST, RIM BOARD C. DOUBLE TOP PLATE LAP AT CORNER C. DOUBLE TOP PLATE LAP AT CORNER C. RIM BOARD, BAND JOIST, OR BLOCK C. RIM BOARD OR RIM JOIST TO JOIST C. BRIDGING OR BLOCKING TO JOIST DESCRIPTION OF BUILDING ELEMEN C. TOR APPLUATION WITHOUT ADHE C. FOR APPLUATION WITHOUT ADHE C. SCREWS FOR ATTACHING GYPSUN WITH ASTM CIOO2 AND SHALL PE TABLE 2	
DESCRIPTION OF BUILD         1. BLOCKING BETWEEN JOISTS OR RAF         2. CEILING JOIST TO TOP PLATE         3. CEILING JOIST TO COLLAR TIE TO RAF         4. CEILING JOIST TO CEILING JOIST (LA         5. RAFTER OR ROOF TRUSS TO TOP PL         6. RAFTER TO RIDGE, VALLEY, OR HIP         7. PACKED OR BUILT-UP STUDS         8. ABUTTING STUDS AT INTERSECTING         9. CONTINUOUS HEADER TO STUD WAI         10. KING STUD TO END OF HEADER         11. TOP PLATE TO TOP PLATE         12. DOUBLE TOP PLATE SPLICE (24" MIN         13. BOTTOM PLATE TO JOIST, RIM BOAR         14. TOP OR BOTTOM PLATE TO STUD         15. DOUBLE TOP PLATE LAP AT CORNER         16. JOIST TO SILL OR DROPPED GIRDER         17. RIM BOARD, BAND JOIST, OR BLOC         18. RIM BOARD OR RIM JOIST TO JOIS         19. LEDGER STRIP SUPPORTING JOIST         20. BRIDGING OR BLOCKING TO JOIST,         DESCRIPTION OF BUILDING ELEMEN         21. ½" GYPSUM         22. 5%" GYPSUM         a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIELL         0. I.77 INCH, AND I OO KSI FOR S         b. MAXIMUM SPACING OF STUDS NC         c. FOR APPLICATION WITHOUT ADHES         PERMITTED TO BE USED WITH THE         d. SCREWS FOR ATTACHING GYPSUN WITH ASTM CIOO2 AND SHALL PE <th>IF ANY CONNECTIONS DESCRIBED IN</th>	IF ANY CONNECTIONS DESCRIBED IN
<ol> <li>BLOCKING BETWEEN JOISTS OR RAF</li> <li>CEILING JOIST TO TOP PLATE</li> <li>CEILING JOIST TO COLLAR TIE TO R/</li> <li>CEILING JOIST TO CEILING JOIST (IA</li> <li>RAFTER OR ROOF TRUSS TO TOP PL</li> <li>RAFTER TO RIDGE, VALLEY, OR HIP</li> <li>PACKED OR BUILT-UP STUDS</li> <li>ABUTTING STUDS AT INTERSECTING</li> <li>CONTINUOUS HEADER TO STUD WAI</li> <li>KING STUD TO END OF HEADER</li> <li>TOP PLATE TO TOP PLATE</li> <li>DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>BOTTOM PLATE TO JOIST, RIM BOA</li> <li>TOP OR BOTTOM PLATE TO STUD</li> <li>DOUBLE TOP PLATE LAP AT CORNER</li> <li>JOIST TO SILL OR DROPPED GIRDEF</li> <li>JEDGER STRIP SUPPORTING JOIST</li> <li>LEDGER STRIP SUPPORTING JOIST</li> <li>LEDGER STRIP SUPPORTING JOIST</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>ALL NAILS ARE SMOOTH-COMMONING MINIMUM AVERAGE BENDING YEL 0.177 INCH, AND 100 KSI FOR S</li> <li>MAXIMUM SPACING OF STUDS NC</li> <li>FOR APPLICATION WITHOUT ADHE PERMITTED TO BE USED WITH THE</li> <li>SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> <li>TABLE 2</li> </ol>	DESCRIPTION OF BUILD
<ol> <li>2. CEILING JOIST TO TOP PLATE</li> <li>3. CEILING JOIST OR COLLAR TIE TO R/</li> <li>4. CEILING JOIST TO CEILING JOIST (IA</li> <li>5. RAFTER OR ROOF TRUSS TO TOP PL</li> <li>6. RAFTER TO RIDGE, VALLEY, OR HIP</li> <li>7. PACKED OR BUILT-UP STUDS</li> <li>8. ABUTTING STUDS AT INTERSECTING</li> <li>9. CONTINUOUS HEADER TO STUD WAI</li> <li>10. KING STUD TO END OF HEADER</li> <li>11. TOP PLATE TO TOP PLATE</li> <li>12. DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>13. BOTTOM PLATE TO JOIST, RIM BOA</li> <li>14. TOP OR BOTTOM PLATE TO STUD</li> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDER</li> <li>17. RIM BOARD, BAND JOIST, OR BLOC</li> <li>18. RIM BOARD OR RIM JOIST TO JOIST</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. <sup>5</sup>/<sub>8</sub>" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMONININUM AVERAGE BENDING YIEL O. 177 INCH, AND IOO KSI FOR SID.</li> <li>MAXIMUM SPACING OF STUDS AN EVEN TH ASTM C IOO2 AND SHALL PE</li> <li>TABLE 2</li> </ol>	I. BLOCKING BETWEEN JOISTS OR RAF
<ol> <li>CEILING JOIST OR COLLAR TIE TO RA</li> <li>CEILING JOIST TO CEILING JOIST (LA</li> <li>RAFTER OR ROOF TRUSS TO TOP PL</li> <li>RAFTER TO RIDGE, VALLEY, OR HIP</li> <li>PACKED OR BUILT-UP STUDS</li> <li>ABUTTING STUDS AT INTERSECTING</li> <li>CONTINUOUS HEADER TO STUD WAI</li> <li>KING STUD TO END OF HEADER</li> <li>TOP PLATE TO TOP PLATE</li> <li>DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>BOTTOM PLATE TO JOIST, RIM BOAN</li> <li>TOP OR BOTTOM PLATE TO STUD</li> <li>DOUBLE TOP PLATE LAP AT CORNER</li> <li>TOP OR BOTTOM PLATE TO JOIST, OR BLOCKING TO JOIST</li> <li>DOUBLE TOP PLATE LAP AT CORNER</li> <li>RIM BOARD, BAND JOIST, OR BLOCKING TO JOIST</li> <li>LEDGER STRIP SUPPORTING JOISTS</li> <li>BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21.</li></ol>	2. CEILING JOIST TO TOP PLATE
<ul> <li>4. CEILING JOIST TO CEILING JOIST (IA</li> <li>5. RAFTER OR ROOF TRUSS TO TOP PL</li> <li>6. RAFTER TO RIDGE, VALLEY, OR HIP</li> <li>7. PACKED OR BUILT-UP STUDS</li> <li>8. ABUTTING STUDS AT INTERSECTING</li> <li>9. CONTINUOUS HEADER TO STUD WAY</li> <li>10. KING STUD TO END OF HEADER</li> <li>11. TOP PLATE TO TOP PLATE</li> <li>12. DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>13. BOTTOM PLATE TO JOIST, RIM BOA</li> <li>14. TOP OR BOTTOM PLATE TO STUD</li> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDEF</li> <li>17. RIM BOARD, BAND JOIST, OR BLOCKING TO JOIST</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. ⅔" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMONING MINIMUM AVERAGE BENDING YIEL O, 177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHER</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	3. CEILING JOIST OR COLLAR TIE TO R
<ul> <li>5. RAFTER OR ROOF TRUSS TO TOP PL</li> <li>6. RAFTER TO RIDGE, VALLEY, OR HIP</li> <li>7. PACKED OR BUILT-UP STUDS</li> <li>8. ABUTTING STUDS AT INTERSECTING</li> <li>9. CONTINUOUS HEADER TO STUD WAI</li> <li>10. KING STUD TO END OF HEADER</li> <li>11. TOP PLATE TO TOP PLATE</li> <li>12. DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>13. BOTTOM PLATE TO JOIST, RIM BOA</li> <li>14. TOP OR BOTTOM PLATE TO STUD</li> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDER</li> <li>17. RIM BOARD OR RIM JOIST, OR BLOG</li> <li>18. RIM BOARD OR RIM JOIST, OR BLOG</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21.  <ul> <li> </li></ul> </li> <li> 21.  <ul> <li> </li> <li> 22.   </li> <li> </li> <li> a. ALL NAILS ARE SMOOTH-COMMONING MINIMUM AVERAGE BENDING YIEL O. 177 INCH, AND IOO KSI FOR S </li> <li> b. MAXIMUM SPACING OF STUDS NC </li> <li> FOR APPLICATION WITHOUT ADHER </li> <li> ALL NATE TO BUILDING FUNDS NC </li> <li> FOR APPLICATION WITHOUT ADHER </li> <li> ALL NAILS ARE SMOOTH-COMMONING MINIMUM SPACING OF STUDS NC </li> <li> FOR APPLICATION WITHOUT ADHER </li> <li> ALL NAILS ARE SMOOTH-COMMONING OF STUDS NC </li> <li> FOR APPLICATION WITHOUT ADHER </li> <li> ALL NAILS ARE SMOOTH-COMMONING OF STUDS NC </li> </ul> </li> </ul>	4. CEILING JOIST TO CEILING JOIST (LA
<ul> <li>AAFTER TO RIDGE, VALLEY, OR HIP</li> <li>7. PACKED OR BUILT-UP STUDS</li> <li>ABUTTING STUDS AT INTERSECTING</li> <li>9. CONTINUOUS HEADER TO STUD WAI</li> <li>10. KING STUD TO END OF HEADER</li> <li>11. TOP PLATE TO TOP PLATE</li> <li>12. DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>13. BOTTOM PLATE TO JOIST, RIM BOA</li> <li>14. TOP OR BOTTOM PLATE TO STUD</li> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDEF</li> <li>17. RIM BOARD OR RIM JOIST, OR BLOC</li> <li>18. RIM BOARD OR RIM JOIST, OR BLOC</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST</li> <li>21.  <ul> <li> </li></ul> </li> <li> 21.   </li> <li> 22.   </li> <li> %" GYPSUM </li> </ul> <li> a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIEL O, 1.77 INCH, AND IO KSI FOR S </li> <li> b. MAXIMUM SPACING OF STUDS NC </li> <li> FOR APPLICATION WITHOUT ADHE PERMITTED TO BE USED WITH THE </li> <li> ASCREWS FOR ATTACHING GYPSUN WITH ASTM CIOO2 AND SHALL PE </li>	5. RAFTER OR ROOF TRUSS TO TOP PL
<ul> <li>7. PACKED OR BUILT-UP STUDS</li> <li>8. ABUTTING STUDS AT INTERSECTING</li> <li>9. CONTINUOUS HEADER TO STUD WAI</li> <li>10. KING STUD TO END OF HEADER</li> <li>11. TOP PLATE TO TOP PLATE</li> <li>12. DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>13. BOTTOM PLATE TO JOIST, RIM BOAN</li> <li>14. TOP OR BOTTOM PLATE TO STUD</li> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDER</li> <li>17. RIM BOARD, BAND JOIST, OR BLOCK</li> <li>18. RIM BOARD OR RIM JOIST TO JOIST</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. ⅔" GYPSUM</li> <li>23. ALL NAILS ARE SMOOTH-COMMONININUM AVERAGE BENDING YIELD ON UNINOUT ADHES</li> <li>b. MAXIMUM SPACING OF STUDS NO</li> <li>c. FOR APPLICATION WITHOUT ADHES</li> <li>PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C 1002 AND SHALL PE</li> </ul>	6. RAFTER TO RIDGE, VALLEY, OR HIP
<ul> <li>ABUTTING STUDS AT INTERSECTING</li> <li>ABUTTING STUD AT INTERSECTING</li> <li>CONTINUOUS HEADER TO STUD WAI</li> <li>IO. KING STUD TO END OF HEADER</li> <li>II. TOP PLATE TO TOP PLATE</li> <li>DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>BOTTOM PLATE TO JOIST, RIM BOA</li> <li>I4. TOP OR BOTTOM PLATE TO STUD</li> <li>I5. DOUBLE TOP PLATE LAP AT CORNER</li> <li>I6. JOIST TO SILL OR DROPPED GIRDER</li> <li>RIM BOARD, BAND JOIST, OR BLOCKING</li> <li>IEDGER STRIP SUPPORTING JOIST</li> <li>IEDGER STRIP SUPPORTING JOIST</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. ¾" GYPSUM</li> <li>ALL NAILS ARE SMOOTH-COMMONIMINIMUM AVERAGE BENDING YIEL O. 177 INCH, AND 100 KSI FOR S</li> <li>MAXIMUM SPACING OF STUDS NC</li> <li>FOR APPLICATION WITHOUT ADHED PERMITTED TO BE USED WITH THE</li> <li>SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	
<ul> <li>2. ADDITING STODES AT INTERSECTING</li> <li>9. CONTINUOUS HEADER TO STUD WAR</li> <li>10. KING STUD TO END OF HEADER</li> <li>11. TOP PLATE TO TOP PLATE</li> <li>12. DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>13. BOTTOM PLATE TO JOIST, RIM BOA</li> <li>14. TOP OR BOTTOM PLATE TO STUD</li> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDER</li> <li>17. RIM BOARD, BAND JOIST, OR BLOCK</li> <li>18. RIM BOARD OR RIM JOIST TO JOIS</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMONININUM AVERAGE BENDING YIEL O. 177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHER PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	7. FACKED OK BUILI-UF STUDS
<ul> <li>I. CONTINUEDED THE DELK TO STOD WAY</li> <li>I. KING STUD TO END OF HEADER</li> <li>I. TOP PLATE TO TOP PLATE</li> <li>I. TOP PLATE TO TOP PLATE SPLICE (24" MIN</li> <li>I.BOTTOM PLATE TO JOIST, RIM BOA</li> <li>I.4. TOP OR BOTTOM PLATE TO STUD</li> <li>I.5. DOUBLE TOP PLATE LAP AT CORNER</li> <li>I.6. JOIST TO SILL OR DROPPED GIRDER</li> <li>I.7. RIM BOARD, BAND JOIST, OR BLOC</li> <li>I.8. RIM BOARD OR RIM JOIST TO JOIST</li> <li>I.1. EDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMONINIMUM AVERAGE BENDING YIEL O. 177 INCH, AND 100 KSI FOR S</li> <li>MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHER PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	9 CONTINUOUS HEADER TO STUD WA
<ul> <li>11. TOP PLATE TO TOP PLATE</li> <li>12. DOUBLE TOP PLATE SPLICE (24" MIN</li> <li>13. BOTTOM PLATE TO JOIST, RIM BOA</li> <li>14. TOP OR BOTTOM PLATE TO STUD</li> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDER</li> <li>17. RIM BOARD, BAND JOIST, OR BLOC</li> <li>18. RIM BOARD OR RIM JOIST TO JOIS</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMONI MINIMUM AVERAGE BENDING YIEL 0.177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHE: PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	10. KING STUD TO END OF HEADER
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<ul> <li>13. BOTTOM PLATE TO JOIST, RIM BOA</li> <li>14. TOP OR BOTTOM PLATE TO STUD</li> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDEH</li> <li>17. RIM BOARD, BAND JOIST, OR BLOC</li> <li>18. RIM BOARD OR RIM JOIST TO JOIS</li> <li>19. LEDGER STRIP SUPPORTING JOISTE</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMONIMINIMUM AVERAGE BENDING YIEL O. 177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHE PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C 1002 AND SHALL PE</li> </ul>	12. DOUBLE TOP PLATE SPLICE (24" MIN
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<ul> <li>15. DOUBLE TOP PLATE LAP AT CORNER</li> <li>16. JOIST TO SILL OR DROPPED GIRDER</li> <li>17. RIM BOARD, BAND JOIST, OR BLOC</li> <li>18. RIM BOARD OR RIM JOIST TO JOIST</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIEL 0.177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC C. FOR APPLICATION WITHOUT ADHER PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	14. TOP OR BOTTOM PLATE TO STUD
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<ul> <li>16. JOIST TO SILL OR DROPPED GIRDEF</li> <li>17. RIM BOARD, BAND JOIST, OR BLOC</li> <li>18. RIM BOARD OR RIM JOIST TO JOIS</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIEL 0. 177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHER PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	
<ul> <li>17. RIM BOARD, BAND JOIST, OR BLOCK</li> <li>18. RIM BOARD OR RIM JOIST TO JOIS</li> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIELI 0.177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHES PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	I G. JOIST TO SILL OR DROPPED GIRDE
<ul> <li>18. RIM BOARD OR RIM JOIST TO JOIS</li> <li>19. LEDGER STRIP SUPPORTING JOIST</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIEL 0.177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHER PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	17. RIM BOARD, BAND JOIST, OR BLO
<ul> <li>19. LEDGER STRIP SUPPORTING JOISTS</li> <li>20. BRIDGING OR BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. ⅔" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMONINIMUM AVERAGE BENDING YIEL 0.177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHES PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	18. RIM BOARD OR RIM JOIST TO JOIS
<ul> <li>20. DRIDGING OK BLOCKING TO JOIST,</li> <li>DESCRIPTION OF BUILDING ELEMEN</li> <li>21. ½" GYPSUM</li> <li>22. 5%" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIEL 0.177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHES PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	19. LEDGER STRIF SUFFORTING JUIST
DESCRIPTION OF BUILDING ELEMEN         21.       Z" GYPSUM         22.       Z" GYPSUM         a.       ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIEL O.177 INCH, AND 100 KSI FOR S         b.       MAXIMUM SPACING OF STUDS NC         c.       FOR APPLICATION WITHOUT ADHE: PERMITTED TO BE USED WITH THE         d.       SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE	ZO. DRIDGING OR DEOCRING TO JOIST,
<ul> <li>21. Z<sup>'</sup> GYPSUM</li> <li>22. Z<sup>'</sup> GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIELD 0.177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHES PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	DESCRIPTION OF BUILDING ELEMEN
<ul> <li>21. Z'' GYPSUM</li> <li>22. Z'' GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIELI O. 177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NCC</li> <li>c. FOR APPLICATION WITHOUT ADHES PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	
<ul> <li>22. ∮<sub>8</sub>" GYPSUM</li> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIEL O. 177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHE: PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	21. Z'' GYPSUM
<ul> <li>a. ALL NAILS ARE SMOOTH-COMMON MINIMUM AVERAGE BENDING YIELI O. 177 INCH, AND 100 KSI FOR S</li> <li>b. MAXIMUM SPACING OF STUDS NC</li> <li>c. FOR APPLICATION WITHOUT ADHES PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	22. 🚀 GYPSUM
<ul> <li>0.177 INCH, AND 100 KSI FOR S</li> <li>MAXIMUM SPACING OF STUDS NC</li> <li>FOR APPLICATION WITHOUT ADHES PERMITTED TO BE USED WITH THE</li> <li>SCREWS FOR ATTACHING GYPSUN WITH ASTM C1002 AND SHALL PE</li> </ul>	a. ALL NAILS ARE SMOOTH-COMMO MINIMUM AVERAGE BENDING YIEL
<ul> <li>c. FOR APPLICATION WITHOUT ADHE: PERMITTED TO BE USED WITH THE</li> <li>d. SCREWS FOR ATTACHING GYPSUN WITH ASTM CIOO2 AND SHALL PE</li> </ul>	0.177 INCH, AND 100 KSI FOR S b. MAXIMUM SPACING OF STUDS NO
PERMITTED TO BE USED WITH THE d. SCREWS FOR ATTACHING GYPSUN WITH ASTM C 1002 AND SHALL PE	c. FOR APPLICATION WITHOUT ADHE
WITH ASTM C 1002 AND SHALL PE	PERMITTED TO BE USED WITH THE
FABLE 2	WITH ASTM CIOO2 AND SHALL PI
	TABLE 2

TABLE

BEAM AND HEADER PLY-TO-PLY ATTACHMENT SCHEDULE IF ANY CONNECTIONS DESCRIBED IN THE PLANS & DETAILS DIFFER FROM WHAT IS SHOWN ON TABLE BELOW, THE PLANS & DETAILS SHALL CONTROL				
DESCRIPTION OF BEAM OR HEADER	NUMBER & TYPE OF FASTENER <sup>a</sup> SPACING			
	BEAMS AND HEADERS NOT IN EXTERIOR WALLS			
I. (2) PLY 2x	(2) ROWS OF I Od NAILS, DRIVEN FROM ONE SIDE	I 2" O.C. IN EACH ROW		
2. (2) PLY LVL	(2) ROWS OF I 6d NAILS, DRIVEN FROM ONE SIDE	I 2" O.C. IN EACH ROW		
3. (3) PLY 2x	(2) ROWS OF I Od NAILS, DRIVEN FROM BOTH SIDES	I 2" O.C. IN EACH ROW		
4. (3) PLY LVL	(2) ROWS OF SIMPSON 5" SDW22 SCREWS, DRIVEN FROM ONE SIDE	I 6" O.C. IN EACH ROW		
5. (4) PLY 2x or LVL	(2) ROWS OF SIMPSON $6^{3_{\rm H}}_4$ SDW22 SCREWS, DRIVEN FROM ONE SIDE	I 6" O.C. IN EACH ROW		
BEAMS AND HEADERS IN EXTER	NOR WALLS (INCREASED FASTENERS FOR COMPOSITE ACTION TO RESIST O.O	.P. WIND LOAD)		
6. (2) PLY 2x	(3) ROWS OF I Od NAILS, DRIVEN FROM ONE SIDE	6" O.C. IN EACH ROW		
7. (2) PLY LVL	(3) ROWS OF I 6d NAILS, DRIVEN FROM ONE SIDE	6" O.C. IN EACH ROW		
8. (3) PLY 2x	(3) ROWS OF I Od NAILS, DRIVEN FROM BOTH SIDES	6" O.C. IN EACH ROW		
9. (3) PLY LVL	(3) ROWS OF SIMPSON 5" SDW22 SCREWS, DRIVEN FROM ONE SIDE	I 2" O.C. IN EACH ROW		
10. (4) PLY 2x or LVL	(3) ROWS OF SIMPSON $6^{3_{\rm H}}_4$ SDW22 SCREWS, DRIVEN FROM ONE SIDE	I 2" O.C. IN EACH ROW		
a. TOP AND BOTTOM ROWS OF FASTENE ROW(S) BETWEEN TOP AND BOTTOM R	RS TO BE LOCATED 2" FROM TOP AND BOTTOM EDGE OF BEAM OR HEADER. OWS.	EQUALLY SPACE MIDDLE		

BEAM/JOIST HANGER AND COLUMN POST BASE/CAP SCHEDULE IF ANY CONNECTIONS DESCRIBED IN THE PLANS & DETAILS DIFFER FROM WHAT IS SHOWN ON TABLE BELOW, THE PLANS & DETAILS SHALL CONTROL					
DESCRIPTION OF BUILDING ELEMENT	TYPE OF CONNECTOR <sup>a</sup>				
JC	NISTS AND BEAMS				
I. I-JOIST	IUS HANGER w/ MATCHING DEPTH AND FLANGE WIDTH				
2. (1) PLY 2x FLOOR JOIST or CEILING JOIST	LUS HANGER w/ MATCHING DEPTH				
3. (2) PLY 2x BEAM	LUS HANGER w/ MATCHING DEPTH				
4. (3) PLY 2x BEAM	HU HANGER w/ MATCHING DEPTH				
5. (I) PLY LVL BEAM	HUS HANGER w/ MATCHING DEPTH				
6. (2) PLY LVL BEAM	HHUS HANGER W/ MATCHING DEPTH				
7. (3) PLY LVL BEAM	HHUS or HGU HANGER w/ MATCHING DEPTH				
8. (4) PLY LVL BEAM	HGUS or HGU HANGER w/ MATCHING DEPTH				
	COLUMNS <sup>b</sup>				
9. BASE FOR 4x, 6x, 8x, 10x, 12x COLUMN	ABU POST BASE w/ MATCHING COLUMN SIZE w/ ANCHOR SIZE PER MANUF.				
I O. CAP FOR 4x, 6x INTERMEDIATE COLUMNS	(2) LPC CAPS PER COLUMN w/ MATCHING WIDTH				
II. CAP FOR 4x, 6x CORNER COLUMNS	(2) LCE4 CAPS PER COLUMN				
a. ALL PREFABRICATED METAL HANGERS AND POST BASES/CAPS	SHALL BE MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY", OR AN				

OR AN FOUIVALENT

TABLE 3

NNG	S ELEMENTS	NUMBER \$ TYPE OF	FASTENER <sup>a</sup>	SPACING AND DESCRIPTION
		ROOF	1	
rers	5 TO TOP PLATE	3 - 8d (2½" x 0.↓↓3")		TOE NAIL
		3 - 8d (2½" x 0.113")		TOE NAIL
FTE	R	5 - 10d (3" x 0.128")		FACE NAIL
PPE	D OVER WALL)	5 - 10d (3" x 0.128")		FACE NAIL
ATE		3 - 10d (3" x 0.128")		TOE NAIL
		5 - 10d (3" x 0.128")		TOE NAIL
		WALL	L	
		Od (3" x 0.   28")		I 2" O.C. FACE NAIL
WAL	L CORNERS	Od (3" x 0.   28")		12" O.C. FACE NAIL
L		4 - 8d (21/2" x 0.     3") PER .	IACK STUD	TOE NAIL
		4 - 16d (3" x 0.128") PER I	PLY OF HEADER	END NAIL
		6d (3½" × 0.135")		I 6" O.C. FACE NAIL
. SF	PLICE LENGTH)	8 - 16d (3.5" x 0.162")		FACE NAIL
RD,	BAND JOIST, OR BLOCKING	6d (3¥2" × 0.  35")		I 6" O.C. FACE NAIL
		4 - 8d (2½" × 0.     3")		TOE NAIL
		2 - 16d (31⁄2" x 0.135")	END NAIL	
5 AN	ND INTERSECTING WALLS	3 - 16d (3.5" x 0.162")		FACE NAIL
	F	LOOR		
<u> </u>		3-8d (21⁄2" x 0.1   3")		TOE NAIL
CKIN	G TO SILL OR TOP PLATE	8d (2½" x 0.     3")		6" O.C. TOE NAIL
T		4 - 10d (3" x 0.128")		END NAIL
OR	RAFTERS	4 - 16d (3½" x 0.135") AT I	EACH STUD	FACE NAIL
RAF	TER, OR TRUSS	4 - 8d (2½" x 0.113") AT EA	CH END	TOE NAIL
			SPACING	GOF FASTENERS
TS	DESCRIPTION O	F FASTENER <sup>a,b,c</sup>	EDGES (INCHES)	INTERMEDIATE SUPPORTS (INCHES)
	GYPSUM BOARD ¢ G	YPSUM PANEL PRODUCTS		
	54 COOLER (0.086"x1 <sup>3</sup> 8" LC WALLBOARD NAIL (0.086"> 0.120" NAILx1 <sup>1</sup> 2" LONG, MII	DNG, $\frac{15}{64}$ " HEAD) OR (1 $\frac{5}{8}$ " LONG, $\frac{3}{32}$ " HEAD) OR N. $\frac{3}{8}$ " HEAD	7	7
NO. 6 TYPE S OR W DRYWALL SCREWS $1\frac{1}{4}$ " LONG <sup>d</sup>		8	12	
	Gd COOLER (0.092"x177" LC WALLBOARD NAIL (0.0915 0.120" NAILx177" LONG, MII	DNG, $\frac{1}{4}$ " HEAD) OR "x I $\frac{7}{6}$ " LONG, $\frac{19}{64}$ " HEAD) OR N. $\frac{3}{6}$ " HEAD	7	7
	NO. 6 TYPE S OR W DRYWALL SCREWS 1 <sup>L</sup> / <sub>4</sub> LONG <sup>d</sup> 8		12	

ESIVE, A PAIR OF NAILS SPACED NOT LESS THAN 2 INCHES APART OR MORE THAN  $2rac{1}{2}$ " APART SHALL BE E PAIR OF NAILS SPACED 12" O.C M BOARD AND GYPSUM PANEL PRODUCTS TO WOOD FRAMING SHALL BE TYPE W OR TYPE S IN ACCORDANCE 'ENETRATE THE WOOD NOT LESS THAN 훓 INCH.

APPROVED EQUAL. INSTALL ALL ACCESSORIES AND FASTENERS PER THE MANUFACTURER'S REQUIREMENTS WITH "MAX" NUMBER OF MANUFACTURER SPECIFIED FASTENERS. NAILS ARE TO BE FULL LENGTH (I OD COMMON = 3" LONG, I GD COMMON =  $3\frac{1}{2}$  LONG). ANCHORS FOR POST BASES ARE TO HAVE A DIAMETER THAT MATCHES SIMPSON SPECIFICATIONS FOR THAT POST BASE. CAST-IN-PLACE ANCHORS ARE TO BE FI 554 GR. 36 HDG L-BOLTS w/ MIN. 7" OF EMBEDMENT. POST INSTALLED ANCHORS (ANCHORS INSTALLED AFTER CONCRETE HAS CURED) ARE TO BE A36 HDG THREADED RODS w/ 7" OF EMBEDMENT SECURED w/ SIMPSON SET, SET-XP, OR SET-3G EPOXY,

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FRIEDENTHAL - GREENE RESIDENCE 906 NORTH PARKWOOD ROAD DECATUR, GEORGIA 30030
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PROJECT NUMBER: 23484
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CHECKED BY: ADAM YOUNG
STAMP
AND PEOJSQ53 * PROFESSIONAL 02/09/2024 FVGINEER STAY H KOBLAS
STRUCTURAL DESIGN NOTES
S-0

![](_page_14_Figure_0.jpeg)

FOUNDATION NOTES:

![](_page_14_Picture_2.jpeg)

\_\_\_\_

FOUNDATION PLAN

FOUNDATION LEG	GEND
EXISTING FOUNDATION WALLS	
COLUMNS	$\boxtimes$
STUD WALLS	

![](_page_14_Figure_6.jpeg)

1. SOLE / SILL PLATES TO BE ANCHORED TO THE FOUNDATION WITH  $/\!\!/_2$  "Ø ANCHOR BOLTS @ A MAXIMUM OF 6'-0" O.C. MINIMUM (2) BOLTS PER PLATE SECTION AND (1) BOLT WITHIN 12" FROM END OF PLATE SECTION. MINIMUM 7" EMBEDMENT INTO MASONRY OR CONCRETE.

SEE SHEET S-0 FOR ADDITIONAL NOTES RELATED TO FOUNDATION. THESE NOTES INCLUDE ALLOWABLE SOIL BEARING CAPACITY, SHORING REQUIREMENTS, AND CONCRETE/REBAR STRENGTH.

![](_page_14_Picture_9.jpeg)

2ND LEVEL FRAMING PLAN

2ND LEVEL FRAMING LEGEND				
	I ST LEVEL WALLS			
	2ND LEVEL WALLS			
	NEW HEADERS OR BEAMS			
	EXISTING HEADERS OR BEAMS			
<>	NEW JOISTS			
←>	EXISTING JOISTS			
$\boxtimes$	COLUMNS BELOW			
$\square$	COLUMNS ABOVE			

![](_page_14_Figure_13.jpeg)

IST LEVEL WALL (BELOW 2ND FLOOR) FRAMING NOTES:

- 1. NEW STUD WALLS TO BE 2x4 @ 16" O.C. w/ 8'-6" MAXIMUM STUD HEIGHT, U.N.O. 2. WINDOW ¢ DOOR HEADERS IN LOAD BEARING WALLS w/ SPANS ≤ 2'-8" SHALL HAVE (1)2x JACK
- STUD; SPANS > 2'-8" SHALL HAVE (2)2x JACK STUDS, U.N.O. 3. WINDOW  $\notin$  DOOR HEADERS IN INTERIOR WALLS SHALL HAVE (1)2x KING STUD; EXTERIOR WALLS w/ SPANS  $\leq 6'-0"$  SHALL HAVE (1)2x KING STUD; EXTERIOR WALLS w/ SPANS > 6'-0" SHALL HAVE
- (2)2x KING STUDS, U.N.O. 4. INTERIOR LOAD BEARING WALLS SHALL HAVE BLOCKING BETWEEN STUDS. DISTANCE BETWEEN BLOCKING PANELS AND BOTTOM/TOP PLATES SHALL NOT EXCEED 5'-0" O.C.
- 5. EXTERIOR WALLS TO BE FULLY SHEATHED w/  $rac{15}{32}$  APA RATED SHEATHING ATTACHED w/ I Od NAILS @ 6" O.C. AT PANEL EDGES # 12" O.C. AT INTERMEDIATE MEMBERS. PROVIDE BLOCKING BETWEEN STUDS AT PANEL EDGES. HORIZONTAL SHEATHING JOINTS SHALL NOT OCCUR AT THE FLOOR LINE, NOR SHALL VERTICAL OR HORIZONTAL JOINTS ALIGN WITH SIDES OF OPENINGS IN THE EXTERIOR WALLS. SHEATHING PANELS SHALL OVERLAP THE FLOOR LINE BY A MINIMUM OF 12 INCHES AND EXTEND A MINIMUM OF 12 INCHES BELOW THE BOTTOM OF THE FLOOR STRUCTURE.

2ND FLOOR FRAMING NOTES:

- 6. ALL NEW FLOOR JOISTS TO BE  $|I_8^Z|$  BCI 60005 @ 16" O.C., U.N.O. 7. FLOOR DECKING TO BE  $\frac{23}{32}$  APA RATED STURD-I-FLOOR 24 OC ADHERED PER JOIST MANUFACTURER TO ENSURE COMPOSITE ACTION & ATTACHED w/ I Od NAILS @ 6" O.C. AT PANEL
- EDGES \$ 12" O.C. AT INTERMEDIATE MEMBERS. 8. WHERE JOISTS ARE PARALLEL TO EXTERIOR WALLS, PROVIDE FULL DEPTH BLOCKING @ 24" O.C. IN I ST BAY TO BRACE WALL.

SEE SHEET S-0 FOR ADDITIONAL NOTES RELATED TO WOOD FRAMING. THESE NOTES INCLUDE WOOD SPECIES/GRADE, HANGER SPECIFICATIONS, AND BEAM PLY ATTACHMENT.

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CHERCISTEREST NO PEO35653 PROFESSIONAL 02/09/2024 CHARLES NGINEER STAL H KOBLAST
FOUNDATION PLAN & 2ND LEVEL FRAMING PLAN
S-1

RELEASED FOR CONSTRUCTION

![](_page_15_Figure_0.jpeg)

SEE SHEET S-O FOR ADDITIONAL NOTES RELATED TO FRAMING. THESE NOTES INCLUDE WOOD SPECIES/GRADE, HANGER SPECIFICATIONS, AND BEAM PLY ATTACHMENT.

WHERE JOISTS ARE PARALLEL TO EXTERIOR WALLS, PROVIDE FULL DEPTH BLOCKING @ 24" O.C. IN

5. EXTERIOR WALLS TO BE FULLY SHEATHED w/  $\frac{15}{32}$ " APA RATED SHEATHING ATTACHED w/ 1 Od NAILS @ 6" O.C. AT PANEL EDGES & 12" O.C. AT INTERMEDIATE MEMBERS. PROVIDE BLOCKING BETWEEN STUDS AT PANEL EDGES. HORIZONTAL SHEATHING JOINTS SHALL NOT OCCUR AT THE FLOOR LINE, NOR SHALL VERTICAL OR HORIZONTAL JOINTS ALIGN WITH SIDES OF OPENINGS IN THE EXTERIOR WALLS. SHEATHING PANELS SHALL OVERLAP THE FLOOR LINE BY A MINIMUM OF 12 INCHES AND EXTEND A MINIMUM OF 12 INCHES BELOW THE BOTTOM OF THE FLOOR STRUCTURE.

3. WINDOW # DOOR HEADERS IN INTERIOR WALLS SHALL HAVE (1)2x KING STUD; EXTERIOR WALLS w/ SPANS  $\leq$  6'-0" SHALL HAVE (1)2x KING STUD; EXTERIOR WALLS w/ SPANS > 6'-0" SHALL HAVE 4. INTERIOR LOAD BEARING WALLS SHALL HAVE BLOCKING BETWEEN STUDS. DISTANCE BETWEEN BLOCKING PANELS AND BOTTOM/TOP PLATES SHALL NOT EXCEED 5'-0" O.C.

I. STUD WALLS TO BE 2x4 @ I 6" O.C. w/ 8'-0" MAXIMUM STUD HEIGHT, U.N.O. 2. WINDOW ¢ DOOR HEADERS IN LOAD BEARING WALLS w/ SPANS ≤ 4'-8" SHALL HAVE (1)2x JACK STUD; SPANS > 4'-8" SHALL HAVE (2)2x JACK STUDS, U.N.O.

\_\_\_\_ \_\_\_\_**>** EXISTING FRAMING to remain \_\_\_\_**/**\_ \_\_\_\_\_ EXISTING FRAMING to remain 

CEILING FRAMING	LEGEND			
2ND LEVEL WALLS				
I ST LEVEL WALLS (OUTSIDE FOOTPRINT OF 2ND LEVEL)				
HEADERS OR BEAMS				
JOISTS	<>			
COLUMNS BELOW	$\boxtimes$			
CEILING DESIGN LOADS				
20 PSF	LIVE LOAD			
I O PSF	LIVE LOAD < 42" ATTIC HEIGHT			
I O PSF	DEAD LOAD			

PACKED STUD SCHEDULE (APPLIES IF COL. IS NOT SPECIFIED ON PLAN)

IF ANY EXISTING CONDITIONS DIFFER FROM WHAT IS SHOWN ON PLAN, CONTACT E.O.R., TYP. ALL SHEETS

(2)PLY BEAM

(3)PLY BEAM

(4)PLY BEAM

(2)2x

(3)2x

(4)2x

![](_page_15_Figure_8.jpeg)

![](_page_15_Picture_9.jpeg)

ROOF FRAMING PLAN

SCALE: |/4'' = |'-0''

SEE SHEET S-O FOR ADDITIONAL NOTES RELATED TO WOOD FRAMING. THESE NOTES INCLUDE WOOD SPECIES/GRADE, HANGER SPECIFICATIONS, AND BEAM PLY ATTACHMENT.

- 5. CONNECT CEILING JOISTS TO RAFTERS w/ A MIN. OF (5)10d NAILS, U.N.O. 6. ONLY BRACE PURLINS ≰ RAFTERS ON CEILING BEAMS OR LOAD BEARING WALLS.
- 4. ALL RAFTERS THAT ARE LABELED "OVER-FRAMING" SHALL BE EITHER BRACED AT THE TOP OR FULLY SHEATHED.

- SUPPORTED EDGES & 12" O.C. AT INTERMEDIATE MEMBERS.
- 2. ALL HIP, VALLEY ≰ RIDGE BOARDS TO BE (1)2x12, U.N.O. 3. ROOF DECKING TO BE  $\frac{15}{32}$ " APA RATED  $\frac{32}{16}$  SHEATHING ATTACHED w/ 10d NAILS @ 6" O.C. AT
- I. ALL NEW RAFTERS TO BE 2x8 @ IG" O.C., U.N.O.

![](_page_15_Figure_21.jpeg)

\_\_\_\_\_

![](_page_15_Figure_22.jpeg)

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KOBLASZ & KENNISON ENGINEERING, PC

> 333 CREEKSTONE RIDGE WOODSTOCK, GEORGIA 30188

ROOF FRAMING LEGEND

EX. ROOF FRAMING

 $\boxtimes$ 

LIVE LOAD

DEAD LOAD

NEW ROOF FRAMING

2ND LEVEL WALLS

I ST LEVEL WALLS

(OUTSIDE FOOTPRINT OF 2ND LEV

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![](_page_16_Figure_0.jpeg)

![](_page_16_Figure_1.jpeg)

![](_page_16_Figure_2.jpeg)

![](_page_16_Figure_3.jpeg)

![](_page_16_Figure_5.jpeg)

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# FRIEDENTHAL - GREENE RESIDENCE

906 NORTH PARKWOOD ROAD DECATUR, GEORGIA 30030

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Contractisty	RET
and the second	35453
* PROFES	SIONAL
02/09/2 EVGU	2024
GARY H	KOBLAS

![](_page_16_Picture_13.jpeg)

![](_page_16_Picture_14.jpeg)