

FLOOD INSURANCE STUDY

VOLUME 1 OF 7



DEKALB COUNTY, GEORGIA AND INCORPORATED AREAS

DeKalb County



COMMUNITY NAME	COMMUNITY NUMBER
ATLANTA, CITY OF	135157
AVONDALE ESTATES, CITY OF	130528
BROOKHAVEN, CITY OF	135175
CHAMBLEE, CITY OF	130066
CLARKSTON, CITY OF	130067
DECATUR, CITY OF	135159
DEKALB COUNTY (UNINCORPORATED AREAS)	130065
DORAVILLE, CITY OF	130069
DUNWOODY, CITY OF	130679
LITHONIA, CITY OF	130472
PINE LAKE, CITY OF	130070
STONE MOUNTAIN, CITY OF	130260

Revised: December 8, 2016



Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER
13089CV001B

**NOTICE TO
FLOOD INSURANCE STUDY USERS**

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study (FIS) may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

Part or all of this FIS may be revised and republished at any time. In addition, part of this FIS may be revised by the Letter of Map Revision process, which does not involve republication or redistribution of the FIS. It is, therefore, the responsibility of the user to consult with community officials and to check the community repository to obtain the most current FIS components.

Initial Countywide FIS Effective Date: May 7, 2001

Revised Countywide FIS Dates: May 16, 2013 – to change Base Flood Elevations and Special Flood Hazard Areas

December 8, 2016 – to change Base Flood Elevations and Special Flood Hazard Areas

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FLOOD INSURANCE STUDY
DEKALB COUNTY, GEORGIA AND INCORPORATED AREAS

1.0 INTRODUCTION

1.1 Purpose of Study

This countywide Flood Insurance Study (FIS) investigates the existence and severity of flood hazards in, or revises previous FISs/Flood Insurance Rate Maps (FIRMs) for, the geographic area of DeKalb County, Georgia, including: the Cities of Atlanta, Chamblee, Clarkston, Decatur, Doraville, Pine Lake, Stone Mountain, Avondale Estates, Lithonia, Dunwoody; and the unincorporated areas of DeKalb County (hereinafter referred to collectively as DeKalb County).

The City of Atlanta is located in Fulton and DeKalb Counties. Flood hazard information for the portion of the City of Atlanta located in Fulton County is included in the FIS for Fulton County, Georgia and Incorporated Areas.

This FIS aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study has developed flood risk data for various areas of the county that will be used to establish actuarial flood insurance rates. This information will also be used by DeKalb County to update existing floodplain regulations as part of the Regular Phase of the National Flood Insurance Program (NFIP), and by local and regional planners to further promote sound land use and floodplain development. Minimum floodplain management requirements for participation in the NFIP are set forth in the Code of Federal Regulations at 44 CFR, 60.3, *Criteria for Land Management and Use*.

In some States or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence and the State (or other jurisdictional agency) will be able to explain them.

1.2 Authority and Acknowledgments

The sources of authority for this FIS are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

This FIS was prepared to include incorporated communities within DeKalb County in a countywide-format report. Information on the authority and acknowledgments for each jurisdiction included in the initial countywide FIS, as compiled from their previously printed FIS reports, is shown on the following pages.

- Atlanta, City of: The hydrologic and hydraulic analyses for South Fork Peachtree Creek and Sugar Creek in the FIS report dated March 4, 1987, were prepared by the U.S. Army Corps of Engineers (USACE), Savannah District, for the Federal Emergency Management Agency (FEMA), under Inter Agency Agreement No. EMW-E-1153, Project Order No. 1. Hydrologic and hydraulic analyses were also taken from the January 3, 1985, FIS for the City of Atlanta, which obtained its hydrologic and hydraulic information from various USACE floodplain information reports (USACE, May 1967, etc.).
- Chamblee, City of: The hydrologic and hydraulic analyses for the FIS report dated September 1977 were performed by the U.S. Geological Survey (USGS), Doraville District, Water Resources Division, for the Federal Insurance Administration (FIA), under Inter-Agency Agreement No. IAA-H-17-75, Project Order No. 15. That work was completed in June 1976.
- Clarkston, City of: The hydrologic and hydraulic analyses for South Fork Peachtree Creek in the FIS report dated December 15, 1980, were performed by the USACE, Savannah District, for FEMA, under Inter-Agency Agreement No. (IAA) H-18-78, Project Order No. 33. That work was completed in October 1979. For the April 17, 1984, FIS report, the hydrologic and hydraulic analyses for South Fork Peachtree Creek and Snapfinger Creek Tributary C were taken from the July 5, 1983, restudy of the unincorporated areas of DeKalb County FIS (FEMA, 1983). The DeKalb County restudy was performed by the USACE, Savannah District, for FEMA under Inter-Agency Agreement No. (IAA) 4-18-78, Project No. 33. That work was completed in October 1980.
- Decatur, City of: The hydrologic and hydraulic analyses for the FIS report dated August 3, 1993, were performed by the USACE, Savannah District for FEMA, under Inter-Agency Agreement No. EMW-89-E-2978, Project Order No. 7. That work was completed in June 1990. The hydrologic and hydraulic analyses for Shoal Creek along the eastern corporate limits were taken from the FIS for the unincorporated areas of DeKalb County, Georgia (FEMA, 1989).

DeKalb County
(Unincorporated
Areas):

The hydrologic and hydraulic analyses for the FIS report dated July 5, 1983, were performed by the USACE, Savannah District, for FEMA under Inter-Agency Agreement No. (IAA) 4-18-78, Project Order No. 33. The mapping and much of the field surveys were performed by John J. Harte Associates, Inc., under contract to the Savannah District. Other survey data were obtained from previous USACE studies. That work was completed in October 1980.

For the FIS report dated July 17, 1986, the hydrologic and hydraulic analyses for South Fork Peachtree Creek were performed by the USACE, Savannah District. For the FIS report dated September 2, 1988, the hydraulic analyses for Henderson Mill Creek and Peachtree Branch were performed by the USACE, Savannah District. For the FIS report dated December 5, 1989, the hydrologic and hydraulic analyses for South Fork Peachtree Creek were performed by McNally and Patrick, Inc. The hydrologic and hydraulic analyses for Nancy Creek Tributary A were performed by Stonehenge Engineering Services Corporation. Supplemental hydraulic analysis for Nancy Creek Tributary A downstream of Village Drive was performed by Mayes, Sudderth, and Etheredge, Inc. The hydrologic and hydraulic analyses for Stone Mountain Creek, Stone Mountain Creek Tributary A, and Crooked Creek Tributary to Stone Mountain Creek were performed by Walden, Ashworth & Associates, Inc. For the FIS report dated June 15, 1994, the hydraulic analyses for Peavine Creek were performed by Travis Pruitt & Associates, P.C.

Doraville, City of:

The hydrologic and hydraulic analyses for the FIS report dated September 15, 1977, were performed by the USGS, Water Resources Division, Doraville, Georgia, for the FIA, under Inter-Agency Agreement No. IAA-H-17-75, Project No. 15. That work was completed in June 1976.

For the FIS report dated August 5, 1985, the hydrologic and hydraulic analyses for Nancy Creek were performed by the USACE, Savannah District, in conjunction with the FIS for the unincorporated areas of DeKalb County (FEMA, 1983).

Pine Lake, City of:

The hydrologic and hydraulic analyses for the FIS report dated December 15, 1980, were performed by the USACE,

Savannah District, for the FIA, under Inter-Agency Agreement No. (IAA)-H-18-78, Project Order No. 33. That work was completed in October 1979.

The authority and acknowledgments for the Cities of Stone Mountain, Avondale Estates, Lithonia, and Dunwoody are not included because there was no previously printed FIS for these communities.

For the May 7, 2001 countywide FIS, the hydrologic and hydraulic analyses for Henderson Mill Creek; Lullwater Creek; Nancy Creek; North Fork Peachtree Creek; North Fork Peachtree Creek Tributaries A, B, C, D-1, D-2, and D-3; Peachtree Branch; Peavine Creek; Perimeter Creek; South Fork Peachtree Creek; and South Fork Peachtree Creek Tributaries A, B, and C were performed by W. L. Jordan & Co., Inc., for FEMA under Contract No. EMA-96-CO-0020. This work was completed in February 1998. The land use delineations were performed for the watershed studied using maps and information supplied by the DeKalb County, Georgia, GIS Department. Topographic mapping for the study areas was prepared by ASI Inc., of Denver, Colorado, under contract with DeKalb County. The use of this topographic information was made possible through an interagency agreement between FEMA and DeKalb County.

The planimetric base map files were provided in digital format by the DeKalb County Geographic Information System Office, The Maloof Center, 1300 Commerce Drive, Room 402, Decatur, Georgia 30030. These files were photogrammetrically compiled by the DeKalb County Geographic Information System Office at a scale of 1: 1,200 from aerial photography dated February 1995.

The digital FIRM was produced in Universal Transverse Mercator coordinates referenced to the North American Datum of 1927 and the Clarke 1866 spheroid.

For the May 16, 2013 revision to the Countywide FIS, the hydrologic and hydraulic analyses were performed by Manhard Consulting Ltd. and Dewberry and Davis LLC, up to the 100-acre drainage limit under contract with DeKalb County Floodplain Management between 2007 and 2011. These watersheds are Barbashela, Blue, Cobbs, Conley, Crooked, Doolittle, Honey/Plunkett, Indian, Intrenchment, Pole Bridge, Shoal, Snapfinger, Stephenson, South River, Sugar and Brookhaven (small watershed tributary to Nancy Creek). Panthers Branch studied within Snapfinger Creek watershed, and Clarks and Corn Creeks studied within South River watershed. Doless Creek studied within Doolittle Creek watershed, and Hardee Creek studied within Sugar Creek watershed. Fowler Branch and Idle Creek studied within Cobbs watershed. Since these studies extended up to the 100-acre drainage area limit, several streams were added to the detailed studies in this FIS report. Refer to Table 3A for Scope of Revision for the Countywide FIS.

1.3 Coordination

An initial Consultation Coordination Officer's (CCO) meeting is held with representatives from FEMA, the community, and the study contractor to explain the nature and purpose of a FIS, and to identify the streams to be studied by detailed methods. A final CCO meeting is held with representatives from FEMA, the community, and the study contractor to review the results of the study.

The dates of the initial and final CCO meetings held for DeKalb County and the incorporated communities within its boundaries are shown in the following tabulation:

<u>Community Name</u>	<u>Initial CCO Date</u>	<u>Final CCO Date</u>
Atlanta, City of	*	January 20, 1995
Chamblee, City of	March and August 1975	September 21, 1976
Clarkston, City of	**	July 11, 1980
Decatur, City of	August 10, 1988	July 9, 1991
DeKalb County (Unincorporated Areas)	May 1978	August 9, 1982
Doraville, City of	March and August, 1975	October 4, 1976
Pine Lake, City of	**	July 11, 1980

* An initial CCO meeting was not held, but the City of Atlanta was notified of the revision by FEMA in a letter dated October 28, 1993.

** An initial CCO meeting was held, but the date of this meeting is not available.

For the May 7, 2001 countywide FIS, streams requiring detailed study or restudy were identified in an initial CCO meeting held on October 26, 1995. This meeting was attended by representatives of FEMA; W.L. Jordan & Co., Inc.; DeKalb County; and representatives from the incorporated communities. A final CCO meeting was held on November 23, 1999. Background data on flooding along the stream corridors was obtained from local officials. Early in the study process, W.L. Jordan & Co., Inc. requested meetings with representatives of all jurisdictional authorities, which would potentially be impacted by the study to determine areas of specific flooding concerns. Authorities responding to these inquiries were met individually to define their concerns. In addition, any planned activities to be developed along the study corridor were requested so that such improvements could be incorporated into the restudy.

For the May 16, 2013 revision, the Multi-Agency Stakeholder Meeting was held on April 5, 2010. The initial CCO meetings were held with DeKalb County and representatives from the incorporated communities at Scott Candler Plant on April 13, 2010 and June 30, 2010. The final CCO meeting was held on October 26, 2011 at Central Transfer Station, 3720 Leroy Scott Drive, Decatur, GA 30032.

2.0 AREA STUDIED

2.1 Scope of Study

The May 7, 2001 countywide FIS covers the geographic area of DeKalb County, Georgia. All or portions of the flooding sources listed in Table 1, "Streams Studied by Detailed Methods," were studied by detailed methods. Limits of detailed study are indicated on the Flood Profiles (Exhibit 1) and on the FIRM (Exhibit 2) where applicable.

Table 1 - Streams Studied by by Detailed Methods

Barbashela Creek	Nancy Creek
Barbashela Creek Tributary A	Nancy Creek Tributary A
Blue Creek	Nancy Creek Tributary B
Burnt Fork Creek	Nancy Creek Tributary C
Camp Creek	Nancy Creek Tributary C-1
Camp Creek Tributary A	Nancy Creek Tributary C-2
Camp Creek Tributary A	Nancy Creek Tributary D
Church Street Branch	Nancy Creek Tributary No. 1
Clarks Creek	Nancy Creek Tributary No. 1.1
Cobbs Creek	North Fork Peachtree Creek
Cobbs Creek Tributary A	North Fork Peachtree Creek Tributary A
Conley Creek	North Fork Peachtree Creek Tributary B
Corn Creek	North Fork Peachtree Creek Tributary B-1
Crooked Creek Tributary to Stone Mountain Creek	North Fork Peachtree Creek Tributary C
Crooked Creek Tributary A	North Fork Peachtree Creek Tributary D-1
Crooked Creek Tributary A-1	North Fork Peachtree Creek Tributary D-2
Crooked Creek (South River)	North Fork Peachtree Creek Tributary D-3
Doless Creek	North Fork Peachtree Creek Tributary No. 1
Doolittle Creek	North Fork Peachtree Creek Tributary No. 1.1
Doolittle Creek Tributary A	North Fork Peachtree Creek Tributary No. 2
Doolittle Creek Tributary B	Panthers Branch
Fowler Branch	Peachtree Branch
Henderson Mill Creek	Peavine Creek
Honey Creek	Peavine Creek Tributary
Honey Creek Tributary A	Perimeter Creek
Indian Creek	Pine Mountain Creek
Indian Creek Tributary A	Pole Bridge Creek
Intrenchment Creek	Pole Bridge Creek Tributary A
Intrenchment Creek Tributary A	Pole Bridge Creek Tributary B
Johnson Creek	Shoal Creek
Lamont Drive Branch	Shoal Creek West Tributary
Little Stone Mountain Creek	Snapfinger Creek
Lullwater Creek	Snapfinger Creek Tributary A
	Snapfinger Creek Tributary B

Table 1 - Streams Studied by Detailed Methods - continued

Snapfinger Creek Tributary C	Sugar Creek
Snapfinger Creek Tributary D	Sugar Creek Tributary
Snapfinger Creek Tributary E	Sugar Creek Tributary A
South Fork Peachtree Creek	Swift Creek
South Fork Peachtree Creek Tributary	Swift Creek Tributary A
South Fork Peachtree Creek Tributary A	Tom George Creek
South Fork Peachtree Creek Tributary B	Yellow River
South Fork Peachtree Creek Tributary C	
South River	
South River Tributary A	
Stephenson Creek	
Stone Mountain Creek	
Stone Mountain Creek Tributary A	

Table 2, "Stream Name Changes," lists streams that have names in the May 7, 2001 countywide FIS other than those used in the previously printed FISs for the communities in which they are located.

Table 2 – Stream Name Changes

<u>Community</u>	<u>Old Name</u>	<u>New Name</u>
Chamblee, City of	Nancy Creek Tributary No. 1	Nancy Creek Tributary C
	Nancy Creek Tributary No. 1.1	Nancy Creek Tributary C-1
	Nancy Creek Tributary No. 1.2	Nancy Creek Tributary C-2
	Nancy Creek Tributary No. 2	Nancy Creek Tributary B
	North Fork Peachtree Creek Tributary No. 1	North Fork Peachtree Creek Tributary A
	North Fork Peachtree Creek Tributary No. 2	North Fork Peachtree Creek Tributary B
	North Fork Peachtree Creek Tributary No. 2.1	North Fork Peachtree Creek Tributary B-1

As part of the May 7, 2001 countywide FIS, updated analyses were included for the flooding sources shown in Table 3, "Scope of Revision for the May 7, 2001 Countywide FIS."

Table 3 – Scope of Revision for May 7, 2001 Countywide FIS

<u>Stream</u>	<u>Limits of New or Updated Detailed Study</u>
Henderson Mill Creek	At the confluence with Peachtree Creek to approximately 0.76 mile upstream of Interstate Route 285

Table 3 – Scope of Revision for May 7, 2001 Countywide FIS – continued

<u>Stream</u>	<u>Limits of New or Updated Detailed Study</u>
Lullwater Creek	At confluence with Peavine Creek to approximately 1,100 feet upstream of Lullwater Parkway crossing
Nancy Creek	At county boundary to approximately 3,680 feet upstream of Tilly Mill Road
North Fork Nancy Creek	At confluence with Nancy Creek to approximately 525 feet upstream of confluence with Nancy Creek
North Fork Peachtree Creek	At the downstream county boundary to approximately 0.7 mile upstream of Pleasantdale Road
North Fork Peachtree Creek Tributary A	At confluence with North Fork Peachtree Creek to approximately 1,600 feet upstream of Tobey Road
North Fork Peachtree Creek Tributary B	At confluence with North Fork Peachtree Creek to approximately 950 feet upstream of Buford Highway
North Fork Peachtree Creek Tributary C	At confluence with North Fork Peachtree Creek to approximately 2,480 feet upstream of Lynnray Drive
North Fork Peachtree Creek Tributary D-1	At confluence with North Fork Peachtree Creek to approximately 900 feet upstream of Greenoaks Circle
North Fork Peachtree Creek Tributary D-2	Approximately 150 feet downstream of Briarcliff Road to approximately 500 feet upstream of Aspen Drive
North Fork Peachtree Creek Tributary D-3	At confluence with North Fork Peachtree Creek Tributary D-1 to approximately 0.4 mile upstream of Greenbrook Way
Peachtree Branch	At confluence with North Fork Peachtree Creek to approximately 1.5 mile upstream of Interstate Route 285
Peavine Creek	At confluence with South Fork Peachtree Creek to Scott Boulevard
Perimeter Creek	At confluence with Nancy Creek to approximately 360 feet upstream of unnamed road

Table 3 – Scope of Revision for May 7, 2001 Countywide FIS – continued

<u>Stream</u>	<u>Limits of New or Updated Detailed Study</u>
South Fork Peachtree Creek	At county boundary to approximately 0.63 mile upstream of Elmdale Road
South Fork Peachtree Creek Tributary A	At confluence with South Fork Peachtree Creek to approximately 0.43 mile upstream of Woburn Drive
South Fork Peachtree Creek Tributary B	At confluence with South Fork Peachtree Creek to approximately 800 feet upstream of Pine Valley Road
South Fork Peachtree Creek Tributary C	At confluence with South Fork Peachtree Creek to approximately 300 feet upstream of North Arcadia Avenue

Additionally, portions of the floodplains of the following streams were revised using updated topographic information:

Barbashela Creek	Snapfinger Creek
Camp Creek	Snapfinger Creek Tributary B
Camp Creek Tributary A	Snapfinger Creek Tributary D
Cobbs Creek	Snapfinger Creek Tributary E
Crooked Creek Tributary A	Indian Creek
Crooked Creek Tributary A-1	Intrenchment Creek
Crooked Creek Tributary to Stone Mountain Creek	Johnson Creek
Doolittle Creek	Little Stone Mountain Creek
Doolittle Creek Tributary A	Nancy Creek Tributary D
Fowler Branch	South Fork Peachtree Creek Tributary
Honey Creek	South River
Honey Creek Tributary A	South River Tributary A
Panthers Branch	Stephenson Creek
Pole Bridge Creek	Stone Mountain Creek
Pole Bridge Creek Tributary A	Swift Creek
Pole Bridge Creek Tributary B	Swift Creek Tributary A
Shoal Creek	Snapfinger Creek
Shoal Creek West Tributary	Yellow River

The areas studied by detailed methods were selected with priority given to all known flood hazard areas and areas of projected development and proposed construction.

Numerous flooding sources in the county were studied by approximate methods. Approximate analyses were used to study those areas having a low development potential or minimal flood hazards. The scope and methods of study were proposed to, and agreed upon by, FEMA and DeKalb County.

As part of the May 16, 2013 countywide FIS revision, new and revised detailed studies were included for the flooding sources shown in Table 3A, "Scope of Revision for May 16, 2013 Countywide FIS Revision."

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Barbashela Creek	From the confluence with Snapfinger Creek to approximately 1,030 feet upstream of Rockborough Drive
Barbashela Creek Tributary A	From the confluence with Barbashela Creek to approximately 250 feet upstream of Megan Road
Barbashela Creek Tributary B	From the confluence with Barbashela Creek to approximately 260 feet upstream of Treehills Parkway
Barbashela Creek Tributary C	From the confluence with Barbashela Creek to approximately 1,060 feet upstream of Mill Lake Circle
Barbashela Creek Tributary D	From the confluence with Barbashela Creek to approximately 2,150 feet upstream of Elam Road
Barbashela Creek Tributary D1	From the confluence with Barbashela Creek Tributary D to approximately 710 feet upstream of Granite Springs Lane
Barbashela Creek Tributary D2	From the confluence with Barbashela Creek Tributary D to approximately 1,070 feet upstream of the confluence with Barbashela Creek Tributary D
Barbashela Creek Tributary E	From the confluence with Barbashela Creek to approximately 2,100 feet upstream of Martin Road
Barbashela Creek Tributary E1	From the confluence with Barbashela Creek Tributary E to approximately 930 feet upstream of Forest Path

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Barbashela Creek Tributary F	From the confluence with Barbashela Creek to approximately 3,830 feet upstream of Selene Drive
Barbashela Creek Tributary G	From the confluence with Barbashela Creek to downstream face of Dam
Barbashela Creek Tributary H	From the confluence with Barbashela Creek to approximately 170 feet downstream of Kenilworth Circle
Barbashela Creek Tributary H1	From the confluence with Barbashela Creek Tributary H to approximately 950 feet upstream of the confluence with Barbashela Creek Tributary H
Barbashela Creek Tributary I	From the confluence with Barbashela Creek to an approximately 390 feet upstream of Sheppard Road
Barbashela Creek Tributary J	From the confluence with Barbashela Creek to approximately 500 feet upstream of West Mountain Street
Blue Creek	From the confluence with South River to approximately 720 feet upstream of Moore Road
Blue Creek Tributary 1	From the confluence with Blue Creek to approximately 740 feet upstream of River Road
Blue Creek Tributary 2	From the confluence with Blue Creek to approximately 870 feet upstream of Bouldercrest Road
Clarks Creek	From the confluence with South River to approximately 720 feet upstream of Brookstone Way
Clarks Creek Tributary 1	From the confluence with Clarks Creek to approximately 760 feet upstream of Brookstone Place
Cobbs Creek	From the confluence with Shoal Creek to an approximately 330 feet upstream of Wiltshire Drive

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Cobbs Creek Tributary A	From the confluence with Cobbs Creek to approximately 50 feet downstream of Cornwall Road
Cobbs Creek Tributary A1	From the confluence with Cobbs Creek Tributary A to approximately 160 feet downstream of Bennington Drive
Cobbs Creek Tributary 1	From the confluence with Cobbs Creek to approximately 60 feet downstream of Stardust Trail
Cobbs Creek Tributary 2	From the confluence with Cobbs Creek to approximately 70 feet downstream of Preakness Drive
Cobbs Creek Tributary 3	From the confluence with Cobbs Creek to approximately 200 feet downstream of Leisure Springs Drive
Cobbs Creek Tributary 4	From the confluence with Cobbs Creek to approximately 260 feet upstream of Rainbow Creek Drive
Cobbs Creek Tributary 5	From the confluence with Cobbs Creek to approximately 80 feet downstream of Spring Valley Road
Cobbs Creek Tributary 6	From the confluence with Cobbs Creek to approximately 480 feet upstream of Oregon Trail
Cobbs Creek Tributary 7	From the confluence with Cobbs Creek to approximately 300 feet upstream of Cindy Drive
Cobbs Creek Tributary 8	From the confluence with Cobbs Creek to approximately 1,300 feet upstream of Wiltshire Drive
Conley Creek	From the confluence with South River to county boundary limit to approximately 710 feet upstream of Thurman Drive

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Conley Creek Tributary 1	From the confluence with Conley Creek to approximately 1,880 feet upstream of the confluence with Conley Creek
Conley Creek Tributary 2	From the confluence with Conley Creek to approximately 200 feet upstream of Seminole Road
Conley Creek Tributary 2A	From the confluence with Conley Creek Tributary 2 to approximately 920 feet upstream of the confluence with Conley Creek Tributary 2
Conley Creek Tributary 3	From the confluence with Conley Creek to approximately 2,930 feet upstream of Ward Lake Road
Conley Creek Tributary 3A	From the confluence with Conley Creek Tributary 3 to approximately 2,720 feet upstream of the confluence with Conley Creek Tributary 3
Conley Creek Tributary 4	From the confluence with Conley Creek to approximately 1,200 feet downstream of Rocky Valley Drive
Conley Creek Tributary 5	From the confluence with Conley Creek to the county boundary limit
Conley Creek Tributary 5A	From the confluence with Conley Creek Tributary 5 to the county boundary limit
Conley Creek Tributary 6	From the confluence with Conley Creek to approximately 290 feet downstream of Highway 675 North Bound Expressway
Conley Creek Tributary 7	From the confluence with Conley Creek to approximately 190 feet upstream of Moreland Avenue
Corn Creek	From the confluence with Clarks Creek to approximately 150 feet downstream of the county boundary limit

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Corn Creek Tributary 1	From the confluence with Corn Creek to approximately 960 feet upstream of the confluence with Corn Creek
Corn Creek Tributary 2	From the confluence with Corn Creek to approximately 560 feet upstream of the confluence with Corn Creek
Crooked Creek (South River)	From the confluence with South River to approximately 5,160 feet upstream of South Goddard Road
Crooked Creek Tributary 1	From the confluence with Crooked Creek to approximately 1,500 feet upstream of Browns Mill Road
Crooked Creek Tributary 2	From the confluence with Crooked Creek to approximately 280 feet upstream of Browns Mill Road
Crooked Creek Tributary 3	From the confluence with Crooked Creek to approximately 360 feet upstream of Browns Mill Road
Crooked Creek Tributary 4	From the confluence with Crooked Creek to approximately 3,000 feet upstream of Roundtree Lane
Crooked Creek Tributary 5	From the confluence with Crooked Creek to approximately 2,050 feet upstream of South Goddard Road
Doless Creek	From the confluence with Doolittle Creek to approximately 120 feet upstream of La Fortune Drive
Doolittle Creek	From the confluence with South River to approximately 20 feet upstream of the Tupelo Street
Doolittle Creek Branch A	From the confluence with Doolittle Creek to approximately 700 feet upstream of Interstate 20

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Doolittle Creek Branch B	From the confluence with Doolittle Creek to approximately 40 feet upstream of Glenwood Avenue
Doolittle Creek Tributary A	From the confluence with Doolittle Creek to approximately 1,130 feet upstream of Rockdale Drive
Doolittle Creek Tributary B	From the confluence with Doolittle Creek to approximately 390 feet upstream of Habersham Drive
Fowler Branch	From the confluence with Cobbs Creek to approximately 1,180 feet upstream of Gretna Green Drive
Fowler Branch Tributary 1	From the confluence with Fowler Branch to approximately 320 feet upstream of Snapfinger Road
Fowler Branch Tributary 2	From the confluence with Fowler Branch to approximately 100 feet downstream of Emerald North Drive
Fowler Branch Tributary 3	From the confluence with Fowler Branch to approximately 40 feet downstream of Lindsey Drive
Fowler Branch Tributary 4	From the confluence with Fowler Branch to approximately 220 feet upstream of Cory Boulevard
Hardee Creek	From the confluence with Sugar Creek to an approximately 300 feet upstream of the Hosea L Williams Drive
Honey Creek	From the county boundary limit to approximately 170 feet upstream of Interstate 20
Honey Creek Tributary A	From the confluence with Honey Creek to approximately 4,470 feet upstream of Forest Lake Parkway

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Idle Creek	From the confluence with Shoal Creek to approximately 850 feet downstream of Wakefield Drive
Indian Creek	From the confluence with Snapfinger Creek to approximately 870 feet upstream of Indian Creek Way
Indian Creek Tributary A	From the confluence with Indian Creek to approximately 2,000 feet downstream of North Clarendon Avenue
Indian Creek Tributary B	From the confluence with Indian Creek to approximately 880 feet upstream of Kensington Road
Indian Creek Tributary C	From the confluence with Indian Creek to approximately 340 feet upstream of Clubhouse Drive
Indian Creek Tributary D	From the confluence with Indian Creek to approximately 380 feet downstream of Woodland Avenue
Indian Creek Tributary D1	From the confluence with Indian Creek Tributary D to approximately 960 feet upstream of Hatton Drive
Indian Creek Tributary D2	From the confluence with Indian Creek Tributary D to an approximately 50 feet downstream of North Decatur Road
Intrenchment Creek	From the confluence with South River to the county boundary limit
Intrenchment Creek Tributary A	From the confluence with Intrenchment Creek to approximately 300 feet upstream of Ormewood Avenue
Intrenchment Creek Tributary 1	From the confluence with Intrenchment Creek to approximately 2,200 feet upstream of the confluence with Intrenchment Creek

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Intrenchment Creek Tributary 2	From the confluence with Intrenchment Creek to approximately 1,040 feet downstream of Eastland Road
Intrenchment Creek Tributary 3	From the confluence with Intrenchment Creek to approximately 140 feet downstream of Moreland Avenue SE
Intrenchment Creek Tributary 4	From the confluence with Intrenchment Creek to approximately 250 feet downstream of Cloverdale Drive
Panthers Branch	From the confluence with Snapfinger Creek to approximately 50 feet downstream of Dividend Drive
Panthers Branch Tributary 1	From the confluence with Panthers Creek to approximately 340 feet downstream of Portsmouth Circle
Panthers Branch Tributary 2	From the confluence with Panthers Creek to approximately 800 feet upstream of Rock Springs Road
Panthers Branch Tributary 3	From the confluence with Panthers Creek to approximately 810 feet upstream of the confluence with Panthers Creek
Plunkett Creek	From the confluence with Honey Creek to approximately 350 feet upstream of Rockland Road
Plunkett Creek Unnamed Tributary	From the confluence with Plunkett Creek to approximately 1,900 feet downstream of Rockland Road
Pole Bridge Creek	From the confluence with South River to approximately 330 feet downstream of Albans Way
Pole Bridge Creek Tributary A	From the confluence with Pole Bridge Creek to approximately 430 feet downstream of Cove Lake Way

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Pole Bridge Creek Tributary AA	From the confluence with Pole Bridge Creek Tributary A to approximately 1,580 feet upstream of Keystone Gate Drive
Pole Bridge Creek Tributary AAA	From the confluence with Pole Bridge Creek Tributary A to approximately 1,070 feet upstream of Shirewick Lane
Pole Bridge Creek Tributary B	From the confluence with Pole Bridge Creek to approximately 40 feet downstream of South Deshon Road
Pole Bridge Creek Tributary BA	From the confluence with Pole Bridge Creek Tributary B to approximately 1,480 feet upstream of Lithonia Industrial Boulevard
Pole Bridge Creek Tributary BAA	From the confluence with Pole Bridge Creek Tributary BA to approximately 530 feet upstream of Tribble Street
Pole Bridge Creek Tributary BB	From the confluence with Pole Bridge Creek Tributary B to approximately 300 feet upstream of Railroad crossing
Pole Bridge Creek Tributary BC	From the confluence with Pole Bridge Creek Tributary B to approximately 2,260 feet downstream of South Deshon Road
Pole Bridge Creek Tributary BD	From the confluence with Pole Bridge Creek Tributary B to Covington Highway
Pole Bridge Creek Tributary C	From the confluence with Pole Bridge to approximately 700 feet downstream of Browns Mill Road
Pole Bridge Creek Tributary D	From the confluence with Pole Bridge Creek to approximately 650 feet downstream of Hunters Valley
Pole Bridge Creek Tributary E	From the confluence with Pole Bridge Creek to approximately 1,300 feet upstream of Hunters Hill Drive

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Pole Bridge Creek Tributary F	From the confluence with Pole Bridge Creek to approximately 260 feet upstream of Timor Trail
Pole Bridge Creek Tributary FA	From the confluence with Pole Bridge Creek Tributary F to approximately 1,280 feet upstream of Ottawa Trail
Pole Bridge Creek Tributary FB	From the confluence with Pole Bridge Creek Tributary F to approximately 600 feet upstream of Winslow Xing
Pole Bridge Creek Tributary G	From the confluence with Pole Bridge Creek to approximately 450 feet upstream of Evans Mill Road
Pole Bridge Creek Tributary H	From the confluence with Pole Bridge Creek to approximately 540 feet upstream of Evans Mill Road
Pole Bridge Creek Tributary HA	From the confluence with Pole Bridge Creek Tributary H to approximately 20 feet upstream of East Glen Road
Pole Bridge Creek Tributary J	From the confluence with Pole Bridge Creek to approximately 1,160 feet downstream of Marbut Road
Pole Bridge Creek Tributary K	From the confluence with Pole Bridge Creek to approximately 800 feet upstream of Marbut Road
Pole Bridge Creek Tributary L	From the confluence with Pole Bridge Creek to approximately 130 feet upstream of Marbut Road
Pole Bridge Creek Tributary LA	From the confluence with Pole Bridge Creek Tributary L to approximately 150 feet upstream of Briar Knoll Road
Pole Bridge Creek Tributary M	From the confluence with Pole Bridge Creek to approximately 1,380 feet upstream of Giles Road
Pole Bridge Creek Tributary N	From the confluence with Pole Bridge Creek to approximately 560 feet upstream of Young Road

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Pole Bridge Creek Tributary O	From the confluence with Pole Bridge Creek to approximately 930 feet upstream of Stoneleigh Hill
Pole Bridge Creek Tributary P	From the confluence with Pole Bridge Creek to approximately 460 feet upstream of Interstate 20
Pole Bridge Creek Tributary Q	From the confluence with Pole Bridge Creek to approximately 230 feet downstream of Spring Lake Overlook
Shoal Creek	From the confluence with South River to the City of Decatur corporate limit
Shoal Creek East Fork Middle Branch	From the confluence with Shoal Creek to approximately 100 feet downstream of Forrest Boulevard
Shoal Creek East Fork Middle Branch Tributary 1	From the confluence with Shoal Creek East Fork Middle Branch to approximately 300 feet upstream of Katie Kerr Drive
Shoal Creek Tributary A	From the confluence with Shoal Creek to approximately 200 feet upstream of Rosewood Road
Shoal Creek Tributary 1	From the confluence with Shoal Creek to approximately 50 feet downstream of Da Vinci Boulevard
Shoal Creek Tributary 2	From the confluence with Shoal Creek to approximately 200 feet downstream of Eastwyck Way
Shoal Creek Tributary 3	From the confluence with Shoal Creek to approximately 830 feet upstream of Miriam Lane
Shoal Creek Tributary 4	From the confluence with Shoal Creek to approximately 40 feet downstream of Nichols Lane

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Shoal Creek Tributary 5	From the confluence with Shoal Creek to approximately 1,760 feet upstream of Meadow Lane
Shoal Creek Tributary 6	From the confluence with Shoal Creek to approximately 580 feet upstream of Shoal Creek Boulevard
Shoal Creek Tributary 7	From the confluence with Shoal Creek to approximately 110 feet upstream of Catalina Drive
Shoal Creek Tributary 8	From the confluence with Shoal Creek to approximately 350 feet downstream of Line Street
Shoal Creek Tributary 9	From the confluence with Shoal Creek to approximately 370 feet upstream of Delano Drive
Snapfinger Creek	From the confluence with South River to approximately 450 feet downstream of East Ponce De Leon Avenue
Snapfinger Creek Tributary A	From the confluence with Snapfinger Creek to approximately 50 feet downstream of Autumn Lake Lane
Snapfinger Creek Tributary A1	From the confluence with Snapfinger Creek Tributary A to approximately 1,100 feet upstream of Sterling Forest Drive
Snapfinger Creek Tributary B	From the confluence with Snapfinger Creek to approximately 70 feet downstream of Interstate 285 Entry Ramp
Snapfinger Creek Tributary B1	From the confluence with Snapfinger Creek Tributary B to approximately 240 feet downstream of Jane Marie Lane
Snapfinger Creek Tributary C	From the confluence with Snapfinger Creek to approximately 350 feet upstream of Reilly Lane

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Snapfinger Creek Tributary C SPLIT	From downstream of Norman Road to approximately 950 feet upstream of Norman Road
Snapfinger Creek Tributary C1	From the confluence with Snapfinger Creek Tributary C to approximately 260 feet upstream of Norman Road
Snapfinger Creek Tributary D	From the confluence with Snapfinger Creek to approximately 490 feet upstream of Central Drive
Snapfinger Creek Tributary E	From the confluence with Snapfinger Creek to approximately 930 feet upstream of Cedar Park Drive
Snapfinger Creek Tributary E1	From the confluence with Snapfinger Creek Tributary E to approximately 190 feet upstream of Cherie Glen Trail
Snapfinger Creek Tributary 1	From the confluence with Snapfinger Creek to approximately 390 feet downstream of Flat Shoals Parkway
Snapfinger Creek Tributary 2	From the confluence with Snapfinger Creek to approximately 1,130 feet upstream of the confluence with Snapfinger Creek
Snapfinger Creek Tributary 3	From the confluence with Snapfinger Creek to approximately 1,890 feet upstream of Snapfinger Woods Drive
Snapfinger Creek Tributary 4	From the confluence with Snapfinger Creek to approximately 800 feet upstream of Interstate 20
Snapfinger Creek Tributary 5	From the confluence with Snapfinger Creek to approximately 230 feet upstream of Cross Lane
Snapfinger Creek Tributary 6	From the confluence with Snapfinger Creek to approximately 120 feet upstream of South Hairston Road

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Snapfinger Creek Tributary 7	From the confluence with Snapfinger Creek to approximately 210 feet upstream of Creekside Place
Snapfinger Creek Tributary 8	From the confluence with Snapfinger Creek to approximately 90 feet downstream of Red Branch Drive
Snapfinger Creek Tributary 9	From the confluence with Snapfinger Creek to approximately 400 feet upstream of Pineford Court
Snapfinger Creek Tributary 9A	From the confluence with Snapfinger Creek Tributary 9 to approximately 240 feet upstream of Young Road
Snapfinger Creek Tributary 9B	From the confluence with Snapfinger Creek Tributary 09 to approximately 580 feet upstream of Greensbrooke Close
Snapfinger Creek Tributary 10	From the confluence with Snapfinger Creek to approximately 1,450 feet upstream of the confluence with Snapfinger Creek
Snapfinger Creek Tributary 11	From the confluence with Snapfinger Creek to approximately 410 feet upstream of Glenhaven Drive
Snapfinger Creek Tributary 12	From the confluence with Snapfinger Creek to approximately 150 feet downstream of To Lani Drive
Snapfinger Creek Tributary 13	From the confluence with Snapfinger Creek to approximately 350 feet upstream of Lake Breeze Drive
Snapfinger Creek Tributary 14	From the confluence with Snapfinger Creek to approximately 650 feet upstream of Allgood Road
Snapfinger Creek Tributary 15	From the confluence with Snapfinger Creek to approximately 170 feet downstream of Woodcrest Manor Drive

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Snapfinger Creek Tributary 16	From the confluence with Snapfinger Creek to approximately 980 feet upstream of the confluence with Snapfinger Creek
Snapfinger Creek Tributary 17	From the confluence with Snapfinger Creek to approximately 280 feet downstream of East Ponce De Leon Avenue
South River	From DeKalb/Rockdale County Boundary, approximately 8,600 feet downstream of Klondike Road to DeKalb/Fulton County Boundary, approximately 1,000 feet upstream of Moreland Avenue
South River Tributary A	From the confluence with South River to approximately 400 feet downstream of Wesley Chapel Road
South River Tributary 1	From the confluence with South River to approximately 2,100 feet upstream of Browns Mill Road
South River Tributary 1A	From the confluence with South River Tributary 01 to approximately 2,900 feet upstream of Browns Mill Road
South River Tributary 1AA	From the confluence with South River Tributary 1A to approximately 1,220 feet upstream of East Saddleridge Drive
South River Tributary 2	From the confluence with South River to approximately 420 feet downstream of Browns Mill Road
South River Tributary 2A	From the confluence with South River Tributary 2 to approximately 130 feet downstream of Panola Lake Circle
South River Tributary 2B	From the confluence with South River Tributary 2 to approximately 630 feet upstream of confluence with South River Tributary 2

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
South River Tributary 3	From the confluence with South River to approximately 2,040 feet upstream of confluence with South River
South River Tributary 4	From the confluence with South River to approximately 1,920 feet upstream of confluence with South River
South River Tributary 5	From the confluence with South River to approximately 2,900 feet upstream of River Road
South River Tributary 5A	From the confluence with South River Tributary 05 to approximately 70 feet upstream of River Road
South River Tributary 6	From the confluence with South River to approximately 500 feet upstream of Cherry Ridge Boulevard
South River Tributary 7	From the confluence with South River to approximately 600 feet upstream of Waldrop Hills Drive
South River Tributary 8	From the confluence with South River to approximately 960 feet upstream of Clifton Springs Road
South River Tributary 9	From the confluence with South River to approximately 430 feet downstream of Bouldercrest Road
South River Tributary 10	From the confluence with South River to approximately 280 feet upstream of Interstate 285 Exit Ramp
South River Tributary 11	From the confluence with South River to approximately 1,200 feet upstream of Fayetteville Road
South River Tributary 12	From the confluence with South River to approximately 120 feet upstream of Moreland Avenue

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
South River Tributary 12A	From the confluence with South River Tributary 12 to approximately 330 feet downstream of Bonsal Road
South River Tributary 14	From the confluence with South River to approximately 1,200 feet upstream of confluence with South River
Stephenson Creek	From the confluence with Pole Bridge Creek to approximately 3,930 feet upstream of Rockland Road
Stephenson Creek Tributary 1	From the confluence with Stephenson Creek to approximately 2,750 feet upstream of South Goddard Road
Stephenson Creek Tributary 2	From the confluence with Stephenson Creek to approximately 2,375 feet upstream of confluence with Stephenson Creek
Sugar Creek	From the confluence with South River to approximately 950 feet upstream of Whitefoord Avenue SE
Sugar Creek East Branch A	From the confluence with Sugar Creek to approximately 1,060 feet downstream of Terry Mill Road
Sugar Creek Tributary A	From the confluence with Sugar Creek to 2 nd Avenue
Sugar Creek Tributary A1	From the confluence with Sugar Creek Tributary A to approximately 840 feet upstream of Lanes Lane
Sugar Creek Tributary C	From the confluence with Sugar Creek to approximately 40 feet upstream of Rollingwood Lane
Sugar Creek Tributary D	From the confluence with Sugar Creek to approximately 200 feet upstream of Creek Crossing Walk

Table 3A - Scope of Revision for May 16, 2013 Countywide FIS Revision – continued

<u>Stream</u>	<u>Limits of New or Revised Detailed Study</u>
Sugar Creek Tributary E	From the confluence with Sugar Creek to approximately 390 feet upstream of Hilburn Drive
Sugar Creek Tributary F	From the confluence with Sugar Creek to approximately 50 feet upstream of Longdale Drive
Sugar Creek Tributary G	From the confluence with Sugar Creek to approximately 50 feet downstream of Clifton Road SE
Sugar Creek Tributary H	From the confluence with Sugar Creek to approximately 60 feet downstream of Blake Avenue
Sugar Creek Tributary I	From the confluence with Sugar Creek to approximately 150 feet upstream of Hardee Circle
Sugar Creek Tributary J	From the confluence with Sugar Creek to approximately 870 feet upstream of Moreland Avenue SE
Sugar Creek Tributary J1	From the confluence with Sugar Creek Tributary J to approximately 70 feet downstream of Hosea L William Drive NE
Unnamed Tributary to Nancy Creek	From approximately 390 feet downstream of Brookhaven Drive to approximately 220 feet upstream of Woodrow Way
Unnamed Tributary 2 to Unnamed Tributary to Nancy Creek	From confluence with Unnamed Tributary to Nancy Creek to approximately 380 feet downstream of Mabry Oaks Drive

The May 16, 2013 countywide FIS also includes re-delineated streams from the effective profiles and an approved LOMR with vertical datum conversion from NGVD 29 to NAVD 88. Table 3B listed the re-delineated streams for the May 16, 2013 countywide FIS revision.

Table 3B - Re-delineated Streams for May 16, 2013 Countywide FIS Revision

Burnt Fork Creek	North Fork Peachtree Creek
Camp Creek	Tributary No. 1.1
Camp Creek Tributary A	North Fork Peachtree Creek
Crooked Creek Tributary A	Tributary No. 2
Crooked Creek Tributary A-1	Peachtree Branch
Crooked Creek Tributary to Stone Mountain Creek	Peavine Creek
Henderson Mill Creek	Peavine Creek Lamont Tributary
Johnson Creek	Peavine Creek Tributary
Little Stone Mountain Creek	Peavine Creek Upland Tributary
Lullwater Creek	Perimeter Creek
Nancy Creek	Pine Mountain Creek
Nancy Creek Tributary A	Shoal Creek (within City of Decatur)
Nancy Creek Tributary B	Shoal Creek Driftwood Tributary
Nancy Creek Tributary C	Shoal Creek Kirk Tributary
Nancy Creek Tributary C-1	Shoal Creek West Tributary
Nancy Creek Tributary C-2	South Fork Peachtree Creek
Nancy Creek Tributary D	South Fork Peachtree Creek Tributary
Nancy Creek Tributary No. 1	South Fork Peachtree Creek Tributary A
Nancy Creek Tributary No. 1.1	South Fork Peachtree Creek Tributary B
Nancy Creek Tributary No. 2	South Fork Peachtree Creek Tributary C
North Fork Nancy Creek	South Fork Peachtree Creek Tributary Glendale Tributary
North Fork Peachtree Creek	South Fork Peachtree Creek Tributary Willow Tributary
North Fork Peachtree Creek Tributary A	South Fork Peavine Creek
North Fork Peachtree Creek Tributary B	Stone Mountain Creek
North Fork Peachtree Creek Tributary B-1	Stone Mountain Creek Tributary A
North Fork Peachtree Creek Tributary C	Sugar Creek Tributary
North Fork Peachtree Creek Tributary D-1	Swift Creek
North Fork Peachtree Creek Tributary D-2	Swift Creek Tributary A
North Fork Peachtree Creek Tributary D-3	Tom George Creek
North Fork Peachtree Creek Tributary No. 1	Yellow River

The May 7, 2001 countywide FIS incorporated the determination of Letters of Map Change issued by FEMA (Letter of Map Revision [LOMR]), as shown in Table 4, "Letters of Map Change."

Table 4 – Letters of Map Change

<u>Community</u>	<u>Flooding Source(s)/Project Identifier</u>	<u>Date Issued</u>	<u>Type</u>
DeKalb County	T. Baker (G&O)	July 3, 1989	LOMR
	Blue Creek at Moore Road	July 9, 1993	LOMR

The May 16, 2013 revision to the countywide FIS incorporates the determination of LOMRs issued by FEMA, as shown in Table 4A, "Letters of Map Revision for the May 16, 2013 Countywide FIS Revision."

Table 4A – Letters of Map Revision for May 16, 2013 Countywide FIS Revision

<u>Community</u>	<u>Case Number</u>	<u>Flooding Source(s)</u>	<u>Date Issued</u>	<u>Date Effective</u>
DeKalb County	10-04-4217P	Peavine Creek	September 1, 2010	October 4, 2010
DeKalb County	10-04-0882P	North Fork Peachtree Creek	July 30, 2010	July 30, 2010
City of Decatur	07-04-3101P	Peavine Creek	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Peavine Creek Lamont Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Peavine Creek Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Peavine Creek Upland Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Shoal Creek	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Shoal Creek Driftwood Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Shoal Creek Kirk Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Shoal Creek West Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	South Fork Peachtree Creek Tributary Glendale Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	South Fork Peachtree Creek Tributary Willow Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Sugar Creek Tributary	October 23, 2007	February 20, 2008
City of Decatur	07-04-3101P	Unnamed Ponding Area	October 23, 2007	February 20, 2008

2.2 Community Description

DeKalb County, located in northwest Georgia, was formed in 1822 from a portion of Henry County. In 1853, a portion of the county, which included the City of Atlanta, was taken to form a part of Fulton County. The land area of the county is now approximately 172,000 acres.

The climate of DeKalb County is temperate with short, usually mild winters and long summers, typical of this part of the United States. Annual average precipitation in the region is 50.2 inches. The average precipitation distribution throughout the year in this region ranges from 3.11 inches in October to nearly 5.38 inches in March (NOAA online weather data, Years 1971-2000). The average annual temperature in DeKalb County (Atlanta Area) is 62.1 degrees Fahrenheit (NOAA online weather data, Years 1971-2000).

Development in the metro-Atlanta area is an important issue in DeKalb County. This area is rapidly evolving into a highly urbanized area with significant growth around established population centers such as Decatur and Atlanta. However, areas of moderate to low development have also begun to develop at an accelerated rate. All indications point to a continuation of this trend throughout the county. The 2010 census indicated a total population within the county boundaries of 691,893 people. In 1995, the Atlanta Regional commission delineated the land use within the county as approximately 25.5 percent urban and 45.6 percent residential. It should be noted that "multi-family residential" was included in the urban classification for land use as this category more closely reflects the land development characteristics associated with that activity. Continuing urbanization of the study area is expected, which will likely lead to intensified development of the floodplain unless these areas are properly identified and managed.

DeKalb County is in the Southern Piedmont major land resource area. The soils on uplands are mainly well drained and have a loamy surface layer and clayey subsoil. The landscape is made up of very gently sloping soils on ridge tops with sloping to steep, well-drained loamy type soil. Although the county is planned mainly for urban use, some excellent farmland exists. Most of the county that has not been developed is in woodland or pastureland. The streams have a steep hydraulic gradient in their headwater reaches, but transition to a moderate gradient as they continue into the major channels.

2.3 Principal Flood Problems

The history of flooding in DeKalb County indicates that flooding may occur during any season of the year, but floods are most likely to occur in winter and spring when runoff conditions are most favorable. Major flood producing storms in these seasons are usually of the frontal type, which last from two to four days and often cover large areas. Although summer storms are usually more intense, they are typically of short duration and limited extent.

There have been numerous flood events in DeKalb County. Significant flood-related damage has been experienced along North Fork Peachtree Creek, South Fork Peachtree Creek, Nancy Creek, their tributaries, and other streams. Past floods have caused damage because of development in the floodplains of the impacted streams. As growth in the county continues, the demand for building sites to accommodate residential and industrial development may force the county to consider sites previously considered unacceptable. Such sites could be vulnerable to serious flooding if proper precautions are not implemented.

2.4 Flood Protection Measures

For the Cities of Atlanta (DeKalb County portion), Chamblee, Clarkston, Doraville, and Pine Lake, there are no existing flood protection measures.

The City of Decatur lies along a drainage divide within the upper catchment areas of four streams. Therefore, the potential for flooding is not as great in the city as it is in developed areas in lower reaches of the catchments. Lower flood flows and narrow floodplains in the upper catchment areas lessen the flood hazard potential. Despite these natural advantages, a number of detached, single-family dwellings and some multi-family structures suffer flood damages because they were built very close to the streams.

The City of Decatur has employed various structural measures to reduce flood damages. Detention ponds have been constructed at Griffin Circle on a branch of Shoal Creek West Tributary and at Montgomery Street on Peavine Creek Tributary. Two detention ponds have been constructed in the Sugar Creek Tributary basin, one north of East Lake Drive and the other east of Second Avenue. Pipes and catch basins have been installed at locations where ponding or erosion has caused problems. The city has made a number of channel improvements, which include replacement, repair, or new construction of culverts, headwalls, and retaining walls. A concrete lining has been installed in the channel of Sugar Creek Tributary between Madison Avenue and Second Avenue. The lining extends east along a branch stream from Second Avenue to a point just east of North Fourth Avenue. One house on Westchester Drive along Peavine Creek Tributary was elevated above the flood level following a storm in May 1980.

DeKalb County completed construction of a detention basin for the Drew Valley Neighborhood in 2009. The detention basin is located just upstream of Drew Valley Road on Poplar Creek, which is a tributary to North Fork Peachtree Creek Tributary A. This watershed had a history of flooding including a flooding event that occurred between June 16 and 17, 2003, which impacted numerous residential structures. During flooding in the area in September 2009, no major flooding issue was reported from the neighborhood.

No other significant channels, structural, or non-structural measures were identified during this study which would reduce potential flood impacts. Although

site specific improvements were noted in some locations, these were usually located at intersections with stream crossings. Further, no proposed stream improvements were identified for development during the study. Any flood protection measures would be maintained by the person or company responsible for their placement rather than the county and would serve to protect an individual property rather than a larger area.

DeKalb County passed an ordinance in 1970 to help protect against significant flooding damages from future flooding events. The ordinance stipulates that all buildings located adjacent to a floodplain drainage easement shall be constructed such that the finished floor elevation of the lowest level is three feet above the regional intermediate flood elevation. Said restriction shall apply to basement floor or crawl areas as well. All streets shall be at least one foot above the regional intermediate flood elevation. Proposed grading or filling operations within a floodplain drainage easement are required to be accompanied by hydraulic calculations which indicate that such activities would not overload the capacity of the channel downstream or increase flood stages upstream. Such demonstration must be made prior to the issue of a permit for said activities. Additionally, calculations must accompany the proposal for development, which indicate that stormwater runoff from the site would not increase by more than one cubic foot per second during a ten-year frequency storm event. Failing such demonstration, a design for on-site storage and controlled discharge of increased stormwater flows must accompany the permit request such that the existing runoff rates for the site are maintained. All drainage improvements shown on approved plans shall be constructed and properly operating before final approval of the development project. All drainage structures and facilities shall be constructed in accordance with county standards and specifications.

3.0 ENGINEERING METHODS

For the flooding sources studied in detail in the county, standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this study. Flood events of a magnitude which are expected to be equaled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long-term average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood which equals or exceeds the 100-year flood (1-percent chance of annual exceedence) in any 50-year period is approximately 40 percent (4 in 10), and, for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the county at the time of completion of

this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish the peak discharge-frequency relationships for each flooding source studied in detail affecting DeKalb County.

Precountywide Analyses

For the unincorporated areas of DeKalb County, City of Atlanta, and the City of Clarkston, the hydrologic analyses were accomplished by computing the peak runoff which would result from storms (rainfall) of a given exceedence frequency. Data from the National Weather Service Technical Paper No. 40 (U.S. Department of Commerce, 1961) were utilized to generate the 10-, 50-, 100- and 500-year hypothetical 24-hour storms. (The rainfall frequency curve was extrapolated to determine the 500-year rainfall). Procedures given in Soil Conservation Service (SCS) Technical Paper 149 (SCS, 1973) were utilized to distribute the rainfall amounts for the hypothetical storms with maximum centering over all subbasins simultaneously for ungaged watersheds. The rainfall distribution, which best recreated recorded flood events in the basin, was used for gaged watersheds.

For the City of Clarkston, there are several gaging stations on South Fork Peachtree Creek, two are in the vicinity of Clarkston, Georgia. Storms recorded on South Fork Peachtree Creek at Williver Drive (drainage area = 11.0 square miles, period of record 1974-1978) were reproduced to calibrate hydrologic model runoff parameters for the area.

The method used to develop discharge frequency estimates for various sites on South Fork Peachtree Creek and Snapfinger Creek Tributary C consisted of computing the estimated peak runoff which would result from storms (rainfall) of a given exceedence frequency. Data from Technical Paper No. 40 (U.S. Department of Commerce, 1963) was utilized to determine the 2-, 10-, 25-, 50-, and 100-year hypothetical 24-hour rainfall. Using rainfall patterns from several severe events in the watershed, a pattern hydrograph was derived from these storm analyses and applied to the hypothetical storms.

For the City of Pine Lake, a crest stage partial record station on Snapfinger Creek at Redan Road, Drainage Area = 13.2 square miles, was in operation during the period 1961, 1963 to 1970. A peak discharge of 3,800 cubic feet per second (cfs), which is estimated to be approximately the 4-year flood, was recorded on May 27, 1963. A frequency analysis was made for comparison with hydrologic model results.

For the City of Pine Lake, the modified PULS stream routing technique was adopted for use in the South Fork Peachtree basin model. The storage outflow

relationships were obtained from a water-surface profile analysis using HEC-2 (USACE, 1991).

The runoff curve number (CN) method described in the Soil Conservation service's National Engineering Handbook, Section 4 (NEH-4), Chapters 7-10 (U.S. Department of Agriculture, 1972), was used to determine the runoff resulting from hypothetical flood events. The curve number method uses the soil series name and type, land use, the slope of the watershed, and the antecedent moisture condition (AMC) in determining the basin's response to runoff, i.e., basin lag. The SCS curve number method has been programmed into the HEC-1 model and is used to develop consistent runoff responses, while allowing the evaluation of the effects of changes in land use. The soil series names were secured from the preliminary soil surveys of DeKalb, Clayton, and Henry Counties, Georgia. The average hydrologic soil group of each subbasin was determined as a weighted average of the area of each subbasin. Rain loss curve numbers were designated for each soil group and a predominant land use associated with each subbasin (Table 5, "DeKalb County Land Use Categories and Associated Curve Numbers (CN)).

Table 5 - DeKalb County Land Use Categories and Associated Curve Numbers (CN)

	USGS Land USE <u>Classification Number</u>	<u>Hydrologic Soil Group</u>			
		<u>A</u>	<u>B</u>	<u>C</u>	<u>D</u>
11	Residential	80	85	90	95
12	Commercial and Industrial	95	96	97	98
13					
15					
*16	Mixed Urban or Built-up Land	73	82	88	91
17	Other Urban or Built-up Land				
21-33	Agricultural and Range Land	67	78	85	89
41-43	Forest Land	36	60	73	79
51-54	Water Bodies	100	100	100	100

*CN from avg. of all urban land uses

Table 5 presents the adopted curve number (CN) associated with each hydrologic soil group and land use for each basin. The relationships shown in Table 5 are based on an intermediate value of antecedent moisture condition (AMC II). Guidance on the adjustment to a drier or wetter condition is given in the SCS Handbook Section 4, Hydrology (NEH-4), Table 10.1, and (U.S. Department of Agriculture, 1972).

Unit hydrographs were computed for each subbasin using HEC-1, along with the generalized dimensionless unit graph described in Chapter 16 of Section 4 of the

SCS Handbook (U.S. Department of Agriculture, 1972). Lag time for each basin hydrograph was determined from the following equation:

$$L = \frac{(l)^{0.8} (S+1)^{0.7}}{1900(Y)^{0.5}} = \frac{(L)^{0.8} (S+1)^{0.7}}{1900(Y)^{0.5}}$$

Where L = hydraulic length of the basin in feet

S = (1000/CN) - 10

CN = Runoff Curve Number

Channel routing for the main water course between subarea combining points was based on the HEC-2 (USACE, 1991) water-surface profile computations, which establishes a relationship between storage and discharge for each channel reach.

Normal depth calculations were used to determine storage-discharge relationship for non-surveyed tributary streams between subbasins. The modified PULS Method from HEC-1 (USACE, 1987) was used to route through each subbasin reach.

Unit hydrographs were computed for each subbasin using HEC-1 with the generalized dimensionless unit hydrographs using Chapter 16 of Section 4 on Hydrology of the National Engineering Handbook (U.S. Department of Agriculture, SCS, 1972).

Channel routing for the main watercourse between subarea combining points was based on the USACE HEC-2 water-surface profile computations, which establish a relationship between storage and discharge for each channel reach (USACE, 1991). Normal depth calculations were used to determine storage discharge relationships for non-surveyed tributary streams between subbasins. The modified PULS Method from HEC-1 was used to route through each subbasin reach.

Flows computed by the rainfall-runoff procedures previously described were verified for ungaged urban streams by using the flood frequency curves for Metro Atlanta (U.S. Department of Interior, USGS, 1977).

Flows for gaged streams were compared to station frequency curves at specific locations on each stream. Because the peak of the hydrograph is attenuated based on storage in the routing reach and the lag time of the hydrograph, peak discharges downstream may be lower than upstream discharges. Timing of local inflows is also a contributing factor.

Peak stage and discharge measurements taken by the USGS, combined with U.S. Weather Bureau records, were used to establish the history of past floods and to compute the heights of probable future floods.

Golden used the Leopold-Sauer technique to develop preliminary flood-frequency relations for urban streams in the Atlanta metropolitan area. The urban relations

were based on regional flood-frequency relationships developed by Golden and Price for small natural streams. The streams studied in detail in Chamblee are within the Atlanta metropolitan area. Therefore, because no gaging records exist for the streams in Chamblee, the hydrologic information from Golden was used (Flood-Frequency Analysis for Small Natural Streams in Georgia).

For the City of Decatur, two methods of hydrologic analyses were carried out to establish the peak discharge-frequency relationships for each riverine flooding source studied in detail affecting the community.

Regional frequency analysis was used to determine peak discharges for Sugar Creek and South Fork Peachtree Creek. Regression equations were adjusted using the Sauer Method to include the effects of urbanization (USGS, 1979; USGS, Water Resources Investigation Report No. 83-4203, 1983). Regression equations developed for the Atlanta Metropolitan area were used to determine peak discharges for Lamont Drive Branch and Church Street Branch (USGS, 1977).

The HEC-1 computer program, including the Soil Conservation Service (SCS) technique for computing unit hydrographs, was used in the calculation of the discharges for Peavine Creek, Peavine Creek Tributary, Shoal Creek, and Shoal Creek West Tributary (USACE, 1987). Hypothetical rainfall data were obtained from the National Weather Service Technical Paper No. 40 for the 10-, 50-, and 100-year events (U.S. Department of Commerce, 1963). The 500-year rainfall was extrapolated from these data. The SCS curve number method was used to determine the amount of runoff resulting from the flood events (U.S. Department of Agriculture, 1972). This method determines the basin lag time and runoff curve number from the hydrologic soil group, land use, and watershed slope. Hydrologic parameters were determined for each subbasin based on an intermediate antecedent moisture condition II (SCS, 1979).

Unit hydrographs, based on a generalized dimensionless unit graph, were computed for each subbasin utilizing HEC-1. Basin lag was determined from the following equation:

$$\text{Lag (hours)} = \frac{L^{0.8} (S+1)^{0.7}}{1900(Y)^{0.5}}$$

Where L = hydraulic length of subbasin in feet
Y = average subbasin land slope in percent
S = (1000/CN) -10
CN = subbasin curve number

For the Cities of Chamblee and Doraville, the discharge-frequency relationships were developed using techniques described by Sauer (U.S. Department of Interior, USGS, 1974) and based on the earlier work by Leopold ("Circular 544,"

1968). Sauer also used the work of Anderson (1970), which suggested that the ratios of various recurrence floods (10-year, 50-year, etc.) to the mean annual flood (2.3-year) for fully developed (100 percent imperviousness) basins have the same ratios as rainfall-intensity data for similar recurrence intervals in that geographic area.

Golden (Preliminary Flood-Frequency Relations for Urban Streams, Metropolitan Atlanta, Georgia) used the Leopold-Sauer technique to develop preliminary flood-frequency relations for urban streams in the Atlanta metropolitan area. The urban relations were based on regional flood-frequency relationships developed by Golden and Price (Flood-Frequency Analysis for Small Natural Streams in Georgia) for small natural streams.

The streams studied in detail in the Cities of Chamblee and Doraville are within the Atlanta metropolitan area. Hydrologic methods used were developed by Golden.

The 10-, 50-, and 100-year recurrence intervals for the streams studied in detail in Chamblee are based on the regional flood discharge data. Peak discharges for the 500-year flood were obtained by straight-line extrapolation of a single-log graph of flood discharges computed for frequencies up to 100 years.

The average frequency-discharge relationships for Nancy Creek Tributaries B, C, C-1, and C-2 are shown in Figure 1, "Frequency-Discharge, Drainage Area Curves," in the City of Chamblee.

The average frequency-discharge relationship for the streams studied in Doraville is shown in Figure 1, "Frequency-Discharge, Drainage Area Curves." The curves were based on average urbanization factors.

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The hydrologic analyses were performed by computing the peak stormwater runoff which would occur as a result of a storm of a given recurrence frequency. Rainfall intensity for these storms was taken from the DeKalb County Drainage Procedures Manual for the 10-, 50-, and 100-year storm events. The 500-year storm intensity was estimated from a log-log projection from the existing storm intensity data. Each storm intensity was based on a 24-hour storm event. The Soil Conservation Services (SCS) document "Technical Release 55," was then used to develop a rainfall distribution pattern. The maximum rainfall intensity was assumed to center over all watershed basins simultaneously during the creation of the hydrologic model.

The watershed of each stream section studied was divided into sub-basins which consisted of similar run-off characteristics or which contributed runoff to the stream section at a given location. The Runoff Curve Number, described in the SCS National Engineering Handbook, was used to describe the runoff conditions

in each of the subbasins. This number takes into consideration the land use and soil types present in the subbasin (U.S. Department of Agriculture, 1972). Soil series present in the study areas were determined by referencing the SCS soil surveys for Gwinnett, DeKalb, and Fulton Counties.

Lag time for each subbasin was calculated by determining the time of concentration for each area and applying the conversion factor of:

$$\text{Lag Time (hours)} = \text{Time of Concentration (minutes)} \times 0.61$$

Time of Concentration was developed by applying a velocity to each sub-section of the hydraulic length of the subbasin. These velocities were determined by considering the slope, land-use in each sub-section and applying the homographs and graphs contained in the DeKalb County Drainage Procedures Manual. These velocities were then applied to the path length to calculate a travel time. The subsection times were then added to determine the total Time of Concentration for the subbasin.

This information combined with antecedent soil moisture content, the slope of the subbasin, and other information is combined to determine the subbasin's response to a storm of a given intensity and duration. The general methodology is often referred to as the "Curve Number Method" of stormwater evaluation. This method is programmed into the HEC-1 E model used during this restudy. This method of evaluation is effective in determining the effects of development on stormwater runoff volumes. The result of these calculations was a unit hydrograph for each subbasin, which was then combined and routed as necessary to model the entire stream section.

Channel routing along the watercourse between points of hydrograph development or combination was based on the Muskingum-Cunge Methodology. This method is incorporated into the HEC-2 hydraulic model and establishes a relationship between discharge and storage for each channel reach. The Muskingum-Cunge Method requires the input of one of three coefficients to perform this calculation. The value of "k" was input for these calculations with the model optimizing the other two coefficients internally. The coefficient "k" is dependent upon the velocity of the water flowing through the channel. The value was originally estimated based on the slope of the channel and following the HEC-2 analyses, the value was optimized using an iterative process.

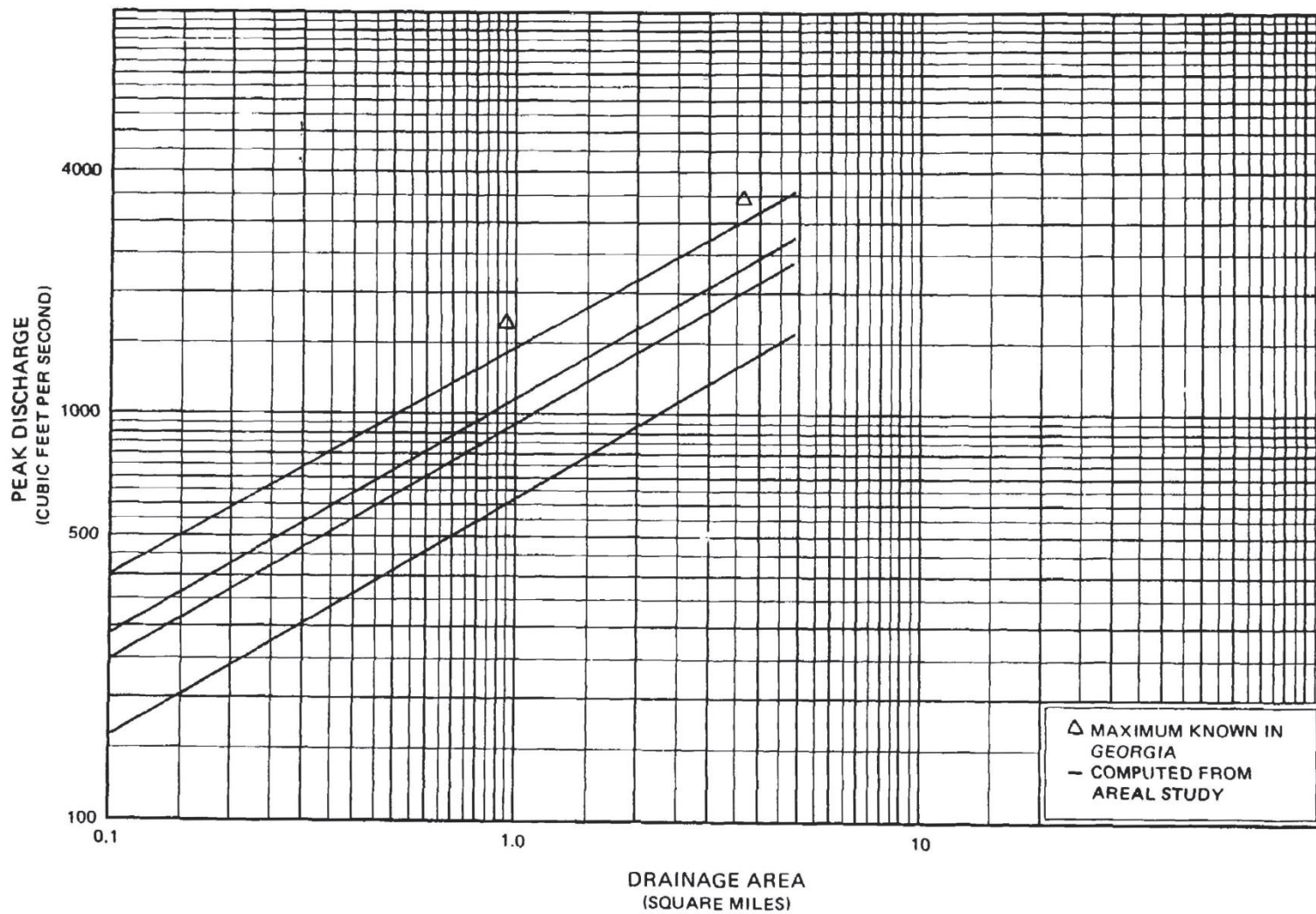


FIGURE 1

FEDERAL EMERGENCY MANAGEMENT AGENCY

**DEKALB COUNTY, GA
AND INCORPORATED AREAS**

FREQUENCY DISCHARGE, DRAINAGE AREA CURVES

ALL STREAMS IN DORAVILLE, GA

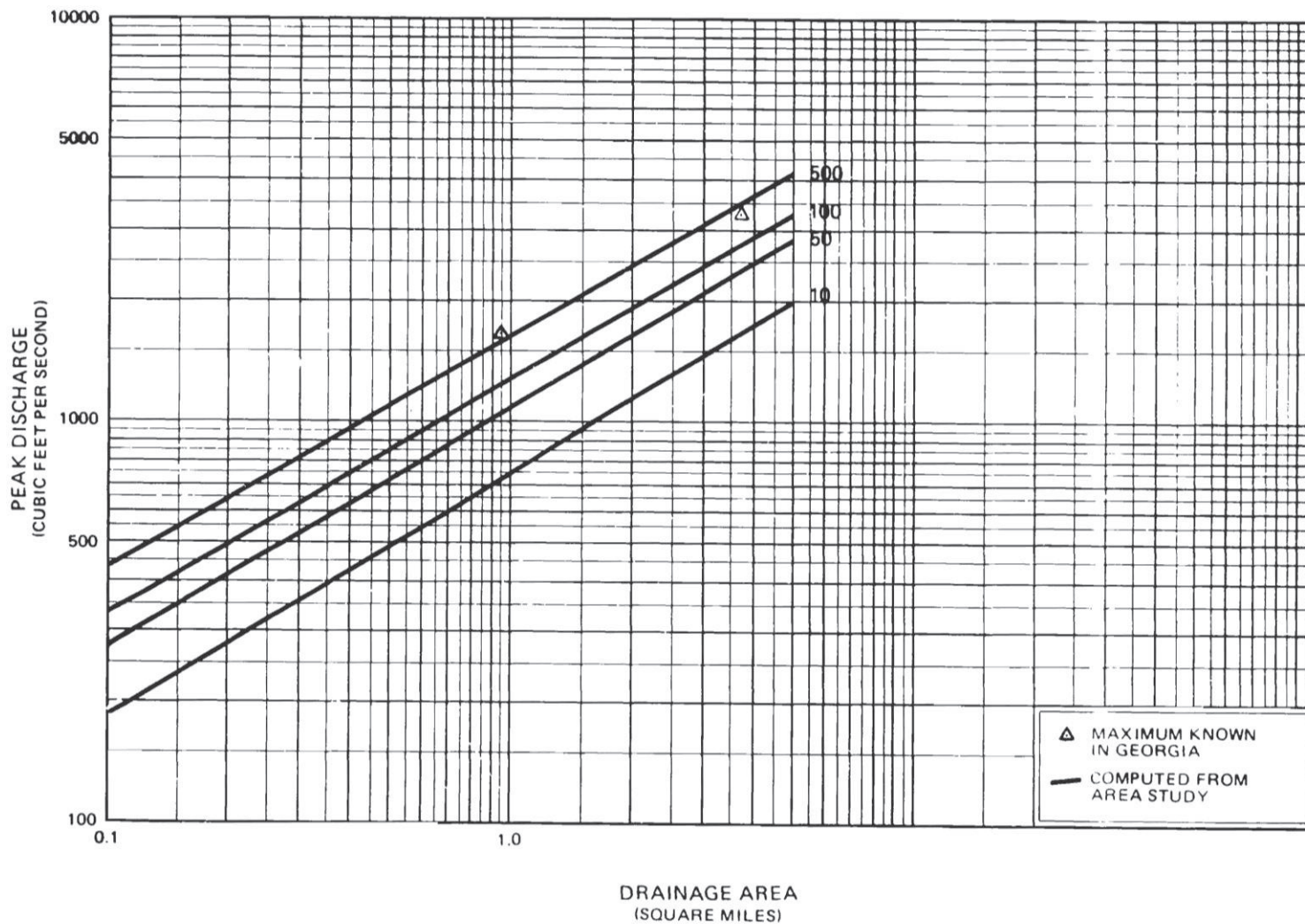


FIGURE 1

FEDERAL EMERGENCY MANAGEMENT AGENCY

**DEKALB COUNTY, GA
AND INCORPORATED AREAS**

FREQUENCY DISCHARGE, DRAINAGE AREA CURVES

**NANCY CREEK TRIBUTARIES C, C-1, C-2, AND B-
NORTH FORK PEACHTREE CREEK TRIBUTARIES A, NO. 2, AND B-1**

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Hydrologic analyses of flood study areas were performed for existing condition to determine peak flow rates. The USACE HEC-Hydrologic Modeling System (HEC-HMS) version 3.3 or higher computer program was used to compute the 2-, 10-, 50-, 100- and 500-year, 24-hour storm events for the project watersheds for existing land-use conditions. The contributing drainage area for each watershed was delineated using DeKalb County's 2-foot interval topography, utilizing the ArcHydro tools, in the Environmental Science and Research Institute's ArcGIS software. The watersheds were subdivided into smaller sub-basins at stream confluences and specific points of interest such as roads, railroads, and dams. The Natural Resources Conservation Service (NRCS) Soil Survey Geographic (SSURGO) digital soils database for DeKalb County was used to determine the hydrologic soil groups within each sub-basin. The existing land-use coverage of the county was used for CN calculation. The land use was categorized into hydrologic land-use codes that correspond with the tables in NRCS Technical Release 55 (TR-55). The hydrologic land-use categories are based on the designations used in NRCS TR-55 (NRCS, 1986), which are typically used in hydrologic studies in estimating runoff curve numbers.

The time of concentration (T_c) for each subbasin was calculated following the methodology described by NRCS TR-55, June 1986. The NRCS three-part approach, called the segmental method, was used to determine the time of concentration parameter for all subbasins within the study areas. The T_c is used to estimate the lag time, which is the parameter input into the HEC-HMS model. The lag time is estimated as $0.6 \times T_c$. The T_c parameter represents the travel time required for stormwater runoff to travel through a basin from the hydraulically most remote point of a watershed to the outlet. In this case, "remoteness" refers to travel time and not travel distance, although these are usually comparable concepts. The segmental method was selected because it provides three physically based flow travel time components to simulate different runoff flow regimes within each subbasin - sheet flow, shallow concentrated flow, and open channel flow. The T_c is the summation of the travel times from each of these flow regimes.

$$\text{Sheet Flow } T_t = \frac{0.007(nL)^{0.8}}{(P_2)^{0.5} S^{0.4}}$$

Where: T_t = travel time for sheet flow in hours

n = effective Manning's n for sheet flow

L = the flow length in feet

P_2 = the 2-year/24-hour rainfall (inches)

S = the slope of the hydraulic grade line as average land slope (ft/ft)

$$\text{Shallow concentrated flow} \quad T = \frac{L}{3600V}$$

Where: T = travel time for shallow concentrated flow in hours

L = the flow length in feet

V = the average runoff velocity 16.1345 (s)^{0.5} for unpaved surfaces,
20.3282(s)^{0.5} for paved surfaces (fps).

$$\text{Open-channel flow} \quad T = \frac{L}{3600V}$$

Where: T = travel time for open-channel flow in hours

L = the flow length in feet

V = Manning's equation for open-channel velocity as shown below

$$V = \frac{1.49 R^{2/3} S^{1/2}}{n}$$

Where: R = hydraulic radius

n = Manning's roughness coefficient

S = the slope of the hydraulic gradient or channel bed slope (ft/ft)

Precipitation was estimated using Table A-12 from the Georgia Stormwater Management Manual (GSMM) Volume 2. This table includes rainfall intensities derived for the Roswell, Georgia, area for durations from 5 to 1,440 minutes (0.08 to 24 hours) and return frequencies from 1- to 100-years (100- to 1-percent annual return frequency). The 1,440-minute intensities were multiplied by 24 to obtain the total rainfall depth for each of the modeled return intervals. The NRCS Type II, 24-hour rainfall distribution was used to develop the design storm hyetographs. The 500-year rainfall depth was estimated by extrapolation.

Storage routing was modeled at existing dams and road structures along the streams, where significant flow attenuation was expected. Elevation-Area and Elevation-Discharge relational tables were developed for all modeled storages. The Elevation-Area relational tables were created with the help of the HEC-GeoRAS tool in ArcGIS, while the Elevation-Discharge relational tables are generated in HEC-RAS by computing multi-flows plan in order to get the elevation-discharge rating curve.

The Muskingum-Cunge channel routing method was used to route surface water from sub-basin outlets. The eight-point cross sections, channel slope, and Manning's 'n' values required for this method were determined from the 2-foot topography, land use, and field survey. The eight-point cross section was used to define typical cross sections of modeled reaches. Channel slope was estimated using the ground surface TIN or raster. Dimensions for the overall cross section were cut from the ground surface TIN or raster and filtered to eight points that most-closely preserves the original geometry. The channel portion of the cross section, defined by the third through sixth point, was manually updated using field measurements. This is necessary because the TIN or raster does not provide

adequate detail on the channel in most cases. Manning's n-values were defined from field observations and aerial photo interpretation. Manning's n-values for channels ranged from 0.035 to 0.055. Overbanks were estimated between 0.06 and 0.15.

A summary of the drainage area-peak discharge for the studied streams by detailed methods for is shown in Table 6, "Summary of Discharges." All revised data from the December 8, 2016, May 16, 2013, and May 7, 2001 FIS reports have been incorporated into the table.

TABLE 6 - SUMMARY OF DISCHARGES

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
BARBASHELA CREEK					
At mouth	9.04	2,703	4,481	5,325	5,954
Approximately 1,880 feet upstream of Woodway Drive	8.69	2,781	4,632	5,517	6,105
At Fieldgreen Drive	7.41	2,698	4,464	5,293	5,802
Approximately 640 feet upstream of Fieldgreen Drive	7.11	2,684	4,437	5,257	5,767
Approximately 1,050 feet downstream of Redan Road	6.66	2,735	4,469	5,272	5,832
At Redan Road	6.56	2,729	4,477	5,297	5,858
Approximately 550 feet downstream of Disney Court	4.14	2,053	3,400	4,027	4,961
Approximately 1,800 feet upstream of Martins Crossing Road	2.51	1,935	2,956	3,418	4,468
Approximately 2,550 feet downstream of Rockbridge Road	2.34	1,967	3,015	3,487	4,547
Approximately 500 feet downstream of Rockbridge Road	1.49	1,752	2,615	2,999	3,857
Approximately 800 feet upstream of Rockbridge Road	1.43	1,775	2,625	3,004	3,857
Approximately 1,200 feet upstream of San Miguel Drive	1.17	1,550	2,267	2,588	3,309
Approximately 150 feet upstream of San Miguel Drive	0.29	379	573	663	876
Approximately 500 feet upstream of Rockborough Drive	0.19	254	393	455	589

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
BARBASHELA CREEK					
TRIBUTARY A					
At mouth	0.92	939	1,441	1,685	2,206
At Fieldgreen Crossing	0.73	815	1,242	1,440	1,857
At Hidden Hills Parkway	0.58	690	1,059	1,217	1,561
At Muirfield Drive	0.31	369	563	649	836
BARBASHELA CREEK					
TRIBUTARY B					
At mouth	0.28	458	654	740	923
BARBASHELA CREEK					
TRIBUTARY C					
At mouth	0.22	64	207	287	538
BARBASHELA CREEK					
TRIBUTARY D					
At mouth	1.29	928	1,893	2,302	2,938
Approximately 625 feet upstream of South Hairston Road	0.64	716	1,217	1,413	1,557
Approximately 620 feet downstream of Elam Road	0.36	547	793	900	1,144
Approximately 820 feet upstream of Elam Road	0.22	369	527	596	744
BARBASHELA CREEK					
TRIBUTARY D1					
At mouth	0.36	455	712	827	1,078
Granite Springs Lane	0.19	260	405	470	611
BARBASHELA CREEK					
TRIBUTARY E					
At mouth	1.01	972	1,624	1,882	1,491
At Willow Run	0.87	938	1,447	1,654	2,094
Approximately 850 feet upstream of Martin Road	0.32	321	497	574	745

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
BARBASHELA CREEK					
TRIBUTARY E1					
At mouth	0.29	330	512	591	767
At Forest Path	0.27	313	486	562	731
BARBASHELA CREEK					
TRIBUTARY F					
At mouth	1.07	458	1,013	1,272	1,428
Approximately 350 feet upstream of Martin Road	0.93	431	966	1,204	1,288
At Selene Drive	0.79	534	1,014	1,195	1,612
BARBASHELA CREEK					
TRIBUTARY F1					
At mouth	0.31	284	443	514	668
Approximately 500 feet downstream of Oshea Lane	0.22	213	330	381	495
BARBASHELA CREEK					
TRIBUTARY G					
At mouth	0.17	41	80	99	145
BARBASHELA CREEK					
TRIBUTARY H					
At mouth	0.56	210	332	386	505
Approximately 150 feet upstream of Rockbridge Road	0.36	44	68	87	150
Approximately 125 feet downstream of Kenilworth Circle	0.29	53	68	86	134
BARBASHELA CREEK					
TRIBUTARY H1					
At mouth	0.18	188	296	344	450
BARBASHELA CREEK					
TRIBUTARY H2					
At mouth	0.21	226	358	416	545

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
BARBASHELA CREEK					
TRIBUTARY I					
At mouth	0.19	281	425	489	626
BARBASHELA CREEK					
TRIBUTARY J					
At mouth	0.84	1,180	1,694	1,930	2,462
Approximately 1,950 feet upstream of Sheppard Road	0.53	846	1,185	1,343	1,706
At West Mountain Street	0.32	554	813	925	1,168
BLUE CREEK					
250 feet Upstream of Oakvale Road	2.21	980	1,392	1,567	1,901
200 feet Downstream of Panthersville Road	2.21	1,320	1,787	1,984	2,386
40 feet Upstream of River Tree Drive	1.93	1,040	1,394	1,532	1,769
800 feet Downstream of Cross Section D	1.93	1,129	1,570	1,747	2,147
230 feet Downstream of Confluence with Tributary 1	1.79	976	1,342	1,478	1,782
50 feet Upstream of River Road	1.57	755	1,158	1,309	1,615
280 feet Downstream of Cross Section H	1.57	836	1,269	1,506	2,164
500 feet Upstream of Cross Section J	1.23	630	1,023	1,239	1,792
880 feet Upstream of Cross Section L	1.15	613	993	1,205	1,747
90 feet Upstream of Cross Section N	1.03	586	952	1,158	1,683
400 feet Upstream of Cross Section O	1.03	676	1,155	1,390	1,961
1,600 feet Downstream of Cross Section Q	0.84	531	910	1,103	1,571
415 feet Upstream of Cross Section P	0.84	880	1,268	1,446	1,850
30 feet Downstream of Confluence with Tributary 2	0.74	746	1,069	1,212	1,581

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
BLUE CREEK (continued)					
50 feet Upstream of Bouldercrest Road	0.52	737	1,048	1,187	1,478
120 feet Upstream of Cross Section X	0.41	626	892	1,007	1,253
690 feet Upstream of Cross Section AA	0.17	391	558	630	784
BLUE CREEK TRIBUTARY 1					
100 feet Downstream of Cross Section A	0.23	328	471	592	817
180 feet Downstream of Cross Section C	0.21	309	445	557	762
700 feet Upstream of Cross Section D	0.21	347	527	606	778
BLUE CREEK TRIBUTARY 2					
220 feet Upstream of Cross Section B	0.22	27	97	161	309
620 feet Upstream of Cross Section C	0.22	310	457	521	659
BURNT FORK CREEK					
Mouth of South Fork Peachtree Creek	5.06	1,528	2,232	2,547	3,367
Approximately 500 feet downstream of Druid Hills Road	4.42	1,570	2,311	2,641	3,493
Near Shamrock High School	3.49	1,875	2,853	3,276	4,344
Approximately 1,000 feet below Frazier Road	2.53	1,665	2,456	2,797	3,653
Approximately 2,000 feet above Montreal Road	1.19	737	1,098	1,257	1,669
Approximately 300 feet above Cooledge Road	0.66	330	519	605	828

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CAMP CREEK					
At the DeKalb/Gwinnett County Line	2.47	1,752	2,579	2,940	3,966
At Confluence with Camp Creek Tributary A	2.37	1,717	2,510	2,859	3,913
Approximately 4,000 feet upstream of Confluence with Camp Creek Tributary A	1.66	1,128	1,684	1,958	2,661
At Cross Section D	1.60	1,091	1,607	1,863	2,557
Approximately 2,100 feet downstream of Antebellum Drive	1.34	1,013	1,405	1,659	2,310
Approximately 480 feet downstream of Antebellum Drive	1.23	999	1,375	1,636	2,254
At Antebellum Drive	1.06	947	1,306	1,553	2,133
CAMP CREEK TRIBUTARY A					
At Royal Woods Parkway	0.71	625	890	990	1,338
Approximately 900 feet downstream of Cross Section G	0.57	539	809	921	1,198
At Summerwalk Parkway	0.39	362	542	614	827
CLARKS CREEK					
130 feet Upstream of Cross Section A	5.63	2,706	4,047	4,596	5,878
1,440 feet Upstream of Hearn Road	5.34	2,730	4,047	4,581	5,878
75 feet Upstream of Lombard Road	4.74	2,704	3,989	4,489	5,834
160 feet Upstream of Lombard Road	1.94	1,305	2,071	2,524	3,519
600 feet Downstream of Cross Section G	1.77	1,234	1,996	2,429	3,383
830 feet Downstream of Cross Section H	1.38	1,005	1,664	2,005	2,741
200 feet Upstream of Scarlett Ohara Drive	1.26	996	1,726	2,083	2,821
40 feet Upstream of Amsler Road	1.13	928	1,604	1,924	2,581

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CLARKS CREEK (continued)					
50 feet Downstream of Confluence with Clarks Creek Tributary 1	0.93	973	1,546	1,802	2,366
300 feet Upstream of Confluence with Clarks Creek Tributary 1	0.76	830	1,308	1,522	1,991
CLARKS CREEK TRIBUTARY 1					
770 feet Upstream of Brookstone Place	0.18	207	331	387	509

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
COBBS CREEK					
At Cross Section B 385 feet Upstream of Cross Section D	9.77	5,373	7,360	8,047	9,341
At Cross Section E 80 feet Downstream of Confluence with Tributary 2	9.60	5,387	7,367	8,045	9,324
1,350 feet Upstream of Cross Section F	9.28	5,354	7,321	8,003	9,258
80 feet Downstream of Confluence with Tributary 3	8.97	5,302	7,253	7,927	9,164
At Cross Section G 20 feet Downstream of Confluence with Tributary 4	8.75	5,269	7,209	7,877	9,102
At Cross Section I	8.58	5,299	7,218	7,871	9,079
At Cross Section K	8.11	5,210	7,106	7,745	8,926
At Cross Section L	8.00	5,272	7,128	7,745	8,910
At Cross Section N 865 feet Downstream of Cross Section P	7.71	5,202	7,049	7,657	8,806
50 feet Downstream of Confluence with Tributary 6	7.51	5,163	6,994	7,588	8,724
425 feet Upstream of Cross Section Q	4.74	3,652	5,177	5,554	6,433
615 feet Upstream of Cross Section T	4.46	3,588	5,078	5,450	6,315
95 feet Upstream of Confluence with Tributary 7	4.24	3,549	5,109	5,644	7,123
At Cross Section V 175 feet Downstream of Confluence with Tributary 9	4.08	3,496	5,025	5,540	7,001
235 feet Upstream of Cross Section AA	3.87	3,434	4,929	5,429	6,865
385 feet Upstream of Cross Section AF	3.72	3,451	4,922	5,396	6,855
At Cross Section AL	3.39	3,348	4,736	5,165	6,626
At Cross Section AO	3.18	3,251	4,600	4,987	6,437
At Cross Section AU 275 feet Upstream of Cross Section AW	2.93	3,472	5,168	5,899	7,463
	2.02	2,163	3,110	3,546	4,533
	1.80	2,012	2,909	3,318	4,250
	1.24	1,565	2,315	2,640	3,414
	0.90	1,112	1,649	1,884	2,444
	0.34	564	846	970	1,239
	0.16	245	372	428	550

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
COBBS CREEK					
TRIBUTARY A					
660 feet Upstream of Cross					
Section F	0.92	1,510	2,274	2,612	3,377
575 feet Downstream of Cross					
Section I	0.78	1,377	2,070	2,374	3,053
At Cross Section J	0.15	365	516	581	720
COBBS CREEK					
TRIBUTARY A1					
850 feet Downstream of Cross					
Section A	0.26	484	738	850	1,094
COBBS CREEK					
TRIBUTARY 1					
900 feet Downstream of Cross					
Section B	0.30	504	769	883	1,137
At Cross Section C	0.21	337	517	597	772
590 feet Upstream of Cross					
Section C	0.21	337	518	598	772
COBBS CREEK					
TRIBUTARY 2					
1,010 feet Upstream of Cross					
Section B	0.23	385	604	701	913
1,610 feet Upstream of Cross					
Section B	0.16	276	434	504	658
COBBS CREEK					
TRIBUTARY 3					
160 feet Downstream of Cross					
Section E	0.47	718	1,113	1,294	1,700
385 feet Upstream of Cross					
Section J	0.38	634	980	1,133	1,472
1,175 feet Upstream of Cross					
Section J	0.20	400	604	693	887
COBBS CREEK					
TRIBUTARY 4					
200 feet Upstream of Cross					
Section B	0.11	237	359	412	527

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
COBBS CREEK					
TRIBUTARY 5					
At Cross Section C 650 feet Upstream of Cross	0.22	248	390	454	593
Section F	0.16	202	318	369	482
COBBS CREEK					
TRIBUTARY 6					
315 feet Downstream of Cross Section A	0.21	327	502	579	746
420 feet Upstream of Cross Section B	0.18	282	433	499	643
COBBS CREEK					
TRIBUTARY 7					
115 feet Upstream of Cross Section B	0.21	354	544	628	810
270 feet Upstream of Cross Section D	0.17	297	453	522	673
COBBS CREEK					
TRIBUTARY 8					
760 feet Upstream of Cross Section D	0.40	328	506	585	766
At Cross Section F	0.21	101	228	283	402
1,225 feet Upstream of Cross Section F	0.20	222	355	414	545
CONLEY CREEK					
500 feet Upstream of Mouth	14.93	3,361	5,191	5,922	8,190
At River Road	14.81	3,369	5,198	5,928	8,192
240 feet Downstream of Confluence with Tributary 1	14.59	3,440	5,315	6,114	8,652
600 feet Downstream of Cross Section D	14.26	3,425	5,289	6,082	8,605
At Cross Section D	14.22	3,446	5,310	6,108	8,634
760 feet Upstream of Cross Section D	14.11	3,440	5,300	6,096	8,617
At Cross Section E	13.95	3,475	5,328	6,137	8,659
600 feet Downstream of Cross Section F	13.40	3,433	5,260	6,056	8,547

TABLE 6 – SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CONLEY CREEK (continued)					
350 feet Upstream of Cross Section F	13.32	3,434	5,257	6,055	8,544
1,050 feet Downstream of Cross Section G	13.17	3,427	5,244	6,040	8,522
340 feet Upstream of Cross Section G	12.99	3,415	5,224	6,017	8,491
75 feet Downstream of Confluence with Tributary 3	12.79	3,432	5,219	6,037	8,509
470 feet Upstream of Cross Section I	10.52	3,259	4,881	5,488	7,648
At Cross Section J	10.14	3,343	4,929	5,511	7,706
1,300 feet Upstream of Ellenwood Road	9.63	3,311	4,953	5,640	9,250
830 feet Downstream of Bouldercrest Road	9.45	3,299	4,857	5,424	7,585
At Cross Section L	9.29	3,289	4,840	5,404	7,558
830 feet Upstream of Cross Section L	9.29	3,407	5,128	5,858	9,496
150 feet Upstream of Confluence with Tributary 5	9.14	3,419	5,133	5,857	9,609
870 feet Downstream of Cross Section M	7.94	3,311	4,953	5,640	9,250
1,260 feet Upstream of Cross Section M	7.60	3,316	4,944	5,618	9,403
800 feet Downstream of Cross Section N	7.34	3,303	4,916	5,579	9,434
1,070 feet Upstream of Cross Section N	7.04	3,291	4,887	5,535	9,499
At Interstate 675	5.52	3,062	4,523	5,092	8,628
660 feet Upstream of Interstate 675	5.36	3,537	5,396	6,220	9,394
620 feet Downstream of Railroad	5.26	3,524	5,376	6,196	9,358
At Railroad	5.13	3,533	5,386	6,209	9,388
At Moreland Avenue	4.78	3,440	5,246	6,045	9,119
2,030 feet Upstream of Moreland Avenue	3.41	2,688	4,046	4,609	6,722
400 feet Downstream of Conley Road	3.01	2,972	4,439	5,068	7,512
At Conley Road	0.50	651	719	765	1,570

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CONLEY CREEK (continued)					
At Thurmond Road	0.29	462	548	699	1,285
270 feet Downstream of Transport City Court	0.29	835	1,157	1,294	1,746
CONLEY CREEK TRIBUTARY 1					
480 feet Upstream of Cross Section A	0.33	430	680	792	1,170
CONLEY CREEK TRIBUTARY 2					
600 feet Upstream of mouth	0.56	637	1,220	1,456	2,181
800 feet Downstream of Cross Section B	0.50	611	1,161	1,381	2,062
540 feet Downstream of Cross Section B	0.32	441	765	897	1,314
190 feet Downstream of Cross Section C	0.26	399	668	775	1,114
At Cross Section D	0.19	358	534	612	869
CONLEY CREEK TRIBUTARY 2A					
At Cross Section A	0.18	370	543	618	868
CONLEY CREEK TRIBUTARY 3					
450 feet Upstream of Mouth	2.28	874	1,302	1,530	2,629
100 feet Downstream of Confluence with Tributary 3A	2.19	864	1,261	1,516	2,610
350 feet Upstream of Confluence with Tributary 3A	1.80	797	1,128	1,407	2,417
At Cross Section C	1.76	793	1,122	1,399	2,404
At Ward Lake Road	1.63	777	1,099	1,375	2,374
400 feet Downstream of Cross Section E	1.58	1,181	1,940	2,329	3,719
480 feet Upstream of Cross Section E	1.45	1,192	1,947	2,352	3,757

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CONLEY CREEK					
TRIBUTARY 3A					
220 feet Upstream of Mouth	0.39	524	823	955	1,407
500 feet Upstream of Cross Section B	0.26	300	489	575	868
CONLEY CREEK					
TRIBUTARY 4					
520 feet Upstream of Mouth	0.51	346	759	1,021	1,789
720 feet Downstream of Cross Section C	0.33	439	746	882	1,342
200 feet Downstream of Rock Knoll Court	0.20	354	557	647	952
CONLEY CREEK					
TRIBUTARY 5					
630 feet Upstream of Mouth	1.17	1,082	1,812	2,116	3,122
290 feet Downstream of Confluence with Tributary 5A	1.09	1,078	1,780	2,069	3,043
1,080 feet Upstream of Cross Section B	0.23	276	447	523	785
CONLEY CREEK					
TRIBUTARY 5A					
650 feet Upstream of Mouth	0.86	823	1,369	1,595	2,380
CONLEY CREEK					
TRIBUTARY 6					
1,010 feet Upstream of Mouth	0.28	306	413	456	592
At East Conley Road	0.19	282	379	412	533
410 feet Upstream of Cross Section E	0.19	325	498	575	831

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CONLEY CREEK					
TRIBUTARY 7					
670 feet Upstream of Mouth	0.35	975	1,331	1,481	1,982
At Cross Section B	0.29	840	1,136	1,262	1,677
90 feet Upstream of Cross Section D	0.29	840	1,028	1,087	1,260
Just Downstream of Moreland Avenue	0.17	538	611	622	646
150 feet Upstream of Moreland Avenue	0.17	538	723	802	1,061
CORN CREEK					
1,740 feet Downstream of Cross Section B	2.80	1,434	2,081	2,390	3,085
540 feet Upstream of Cross Section C	2.47	1,356	1,977	2,288	2,947
50 feet Downstream of Confluence with Corn Tributary 1	2.01	1,125	1,711	1,948	2,538
240 feet Upstream of Confluence with Corn Tributary 2	1.71	1,023	1,603	1,841	2,412
460 feet Downstream of Cross Section F	1.52	964	1,510	1,747	2,282
CORN TRIBUTARY 1					
750 feet Upstream of Cross Section A	0.15	226	343	395	507
CORN TRIBUTARY 2					
380 feet Upstream of Cross Section A	0.19	249	391	454	593
CROOKED CREEK (SOUTH RIVER)					
450 feet Upstream of Confluence with South River	4.00	1,504	2,453	3,028	4,419
2,800 feet Upstream of Confluence with South River	3.81	1,477	2,405	2,979	4,384
At Cross Section B	3.52	1,323	2,246	2,800	4,189

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CROOKED CREEK (SOUTH RIVER) (continued)					
550 Upstream of Confluence with Tributary 1	2.71	1,042	1,522	2,071	3,300
At Cross Section D	2.51	1,009	1,477	2,010	3,214
250 feet Downstream of Browns Mill Road	2.41	1,450	2,020	2,252	3,549
At Browns Mill Road	1.79	886	1,314	1,896	3,078
180 feet Upstream of Browns Mill Road	1.79	960	1,492	2,035	3,154
At White Circle	1.76	956	1,487	2,030	3,151
At Cross Section K	1.76	1,216	2,162	2,597	3,619
400 feet Downstream of Confluence with Tributary 4 and 5	1.56	1,241	2,279	2,790	3,915
At S. Goddard Road	0.79	616	1,104	1,315	1,802
At Cross Section O	0.79	925	1,352	1,542	1,974
At Cross Section P	0.53	626	911	1,037	1,313
Pond 3,700 feet Upstream of S. Goddard Road	0.27	41	107	149	280
1,400 feet Upstream of Pond	0.27	779	1,091	1,226	1,518
CROOKED CREEK (SOUTH RIVER) TRIBUTARY 1					
1,200 feet Upstream of Confluence with Crooked Creek	0.78	684	1,014	1,161	1,523
At Cross Section B	0.70	541	807	931	1,292
1,000 feet Downstream of Browns Mill Road	0.70	541	805	928	1,282
At Browns Mill Road	0.38	51	195	277	508
Pond 400 feet Upstream of Browns Mill Road	0.36	45	190	270	497
1,000 feet Upstream of Pond Outlet	0.36	582	882	1,019	1,327
CROOKED CREEK (SOUTH RIVER) TRIBUTARY 2					
At Cross Section D	0.27	455	609	669	776
At Browns Mill Road	0.20	348	457	490	548

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CROOKED CREEK (SOUTH RIVER) TRIBUTARY 2					
(continued)					
At Cross Section D	0.27	455	609	669	776
At Browns Mill Road	0.20	348	457	490	548
300 feet Upstream of Browns Mill Road	0.20	383	566	648	830
CROOKED CREEK (SOUTH RIVER) TRIBUTARY 3					
400 feet Upstream of Confluence with Crooked Creek	0.31	529	809	931	1,228
300 feet Upstream of Browns Mill Road	0.22	425	631	723	927
CROOKED CREEK (SOUTH RIVER) TRIBUTARY 4					
200 feet Upstream of Confluence with Crooked Creek	0.46	576	835	1,029	1,438
At Roundtree Lane	0.45	571	827	1,020	1,420
500 feet Upstream of Roundtree Lane	0.45	598	938	1,088	1,427
At Cross Section C	0.37	515	797	924	1,206
300 feet Upstream of Cross Section D	0.17	286	435	502	654
CROOKED CREEK (SOUTH RIVER) TRIBUTARY 5					
At S. Goddard Road	0.19	77	273	380	609
At Cross Section D	0.18	74	265	371	593
1,600 feet Upstream of Pond Outlet	0.18	368	543	622	795

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CROOKED CREEK					
TRIBUTARY TO STONE					
MOUNTAIN CREEK					
At mouth	8.71	1,205	2,217	2,819	4,634
At dam downstream of Deshon Road	8.38	1,513	2,642	3,281	5,175
At Deshon Road	7.81	1,403	2,441	3,047	4,785
At upstream of Deshon Road	7.69	1,383	2,405	3,004	4,710
Above confluence of Crooked Creek Unnamed Tributary 1	6.69	1,157	2,137	2,702	4,391
At Stephenson Road	6.57	1,152	2,132	2,695	4,380
At Approximately 1,670 feet downstream of Alford Road	6.00	1,118	2,078	2,625	4,256
At Alford Road	5.72	1,098	2,048	2,586	4,185
At Carriage Trace Circle	5.36	1,071	2,004	2,528	4,075
Above confluence of Crooked Creek Tributary A	2.42	681	1,283	1,543	2,362
At Greenock Drive	2.29	734	1,323	1,569	2,371
Above confluence of Crooked Creek Unnamed Tributary 2	1.93	621	1,126	1,358	2,087
Above confluence of Crooked Creek Unnamed Tributary 3	1.04	591	1,021	1,198	1,748
At Approximately 1,150 feet downstream of Simone Way	0.51	501	805	954	1,315
At Simone Way	0.41	442	712	839	1,157
At dam downstream of Longbow Drive	0.20	239	381	446	607
CROOKED CREEK					
TRIBUTARY A					
At mouth	2.86	397	755	1,035	1,851
At Mystery Valley Dam	2.75	383	740	1,017	1,824
At Nell Yates Drive	0.84	436	715	846	1,297
At Shadow Lakes Drive	0.72	366	589	693	1,075
At dam south of Kingway Drive	0.52	235	355	419	669
At Shadow Rock Drive	0.34	83	195	264	471

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

Peak Discharges (cubic feet per second)

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
CROOKED CREEK					
TRIBUTARY A-1					
At mouth	1.77	921	1,540	1,817	2,646
At Valley Green Road	1.29	527	944	1,160	1,944
At Shadow Rock Drive	0.82	242	573	752	1,270
At detention basin approximately 1,500 feet upstream of Shadow Rock Drive	0.51	158	425	540	813
DOLESS CREEK					
At mouth	1.31	1,502	1,850	1,971	2,261
At Interstate Route 285	1.11	1,451	2,147	2,394	2,919
At approximately 1,000 feet upstream of Flat Shoals Road	0.53	809	1,144	1,233	1,408
At Interstate Route 20	0.37	466	677	716	784
DOOLITTLE CREEK					
At mouth	7.06	3,174	5,243	5,934	7,435
At Clifton Springs Road	5.59	2,454	3,708	4,211	5,291
At Interstate Route 285	5.29	2,833	4,187	4,785	6,028
At Doolittle Creek Tributary A confluence	4.28	2,249	3,288	3,760	4,790
At Doolittle Creek Branch A confluence	3.67	1,805	2,562	2,929	3,619
At Interstate Route 20	3.33	1,971	3,169	3,710	4,969
Above Doolittle Creek Tributary B confluence	2.47	1,528	2,366	2,745	3,768

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
DOOLITTLE CREEK (continued)					
Above Doolittle Creek East					
Branch B confluence	1.50	747	1,213	1,421	1,857
At McAfee Road	1.15	398	642	752	1,005
At Glenwood Avenue	0.82	589	951	1,113	1,471
DOOLITTLE CREEK BRANCH A					
At mouth	0.33	475	754	878	1,151
DOOLITTLE CREEK EAST BRANCH B					
At mouth	0.83	661	985	1,127	1,695
At McAfee Road	0.48	338	495	631	905
At Marion Circle	0.33	390	611	709	927
DOOLITTLE CREEK TRIBUTARY A					
At mouth	0.44	446	675	764	945
At Tyler Way	0.26	275	419	483	621
DOOLITTLE CREEK TRIBUTARY B					
At mouth	0.69	890	1,386	1,606	2,089
At Tyler Way	0.43	475	754	878	1,151
FOWLER BRANCH					
At Cross Section B	2.77	2,585	3,936	4,540	5,896
310 feet Downstream of Cross Section C	2.64	2,627	3,966	4,560	5,876
At Cross Section C	2.60	2,665	4,015	4,619	5,947
At Cross Section D	2.27	2,199	3,336	3,844	4,982
Downstream of Confluence of Fowler Branch Tributary 2	2.22	2,177	3,300	3,798	4,923
125 feet Downstream of Cross Section J	1.72	1,666	2,500	2,879	3,720
135 feet Upstream of Cross Section O	1.48	1,479	2,233	2,573	3,319
200 feet Upstream of Cross Section Q	1.22	1,130	1,716	1,978	2,555

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
FOWLER BRANCH					
(continued)					
290 feet Downstream of					
Cross Section T	0.97	830	1,248	1,434	1,839
At Cross Section T	0.85	772	1,155	1,324	1,685
210 feet Upstream of					
Cross Section W	0.59	7459	755	890	1,183
At Cross Section BB	0.26	188	288	328	419
850 feet Upstream of					
Cross Section BB	0.26	562	817	928	1,167
FOWLER BRANCH					
TRIBUTARY 1					
650 feet Upstream of Mouth	0.33	612	874	988	1,235
225 feet Downstream of					
Cross Section A	0.32	605	861	972	1,214
255 feet Upstream of					
Cross Section D	0.16	392	534	594	722
FOWLER BRANCH					
TRIBUTARY 2					
385 feet Upstream of					
Cross Section B	0.33	404	747	897	1,207
910 feet Upstream of					
Cross Section B	0.32	378	716	861	1,161
1,360 feet Upstream of					
Cross Section B	0.18	295	463	538	702
1,810 feet Upstream of					
Cross Section B	0.16	265	415	482	629
FOWLER BRANCH					
TRIBUTARY 3					
350 feet Downstream of					
Cross Section A	0.23	325	493	567	729
275 feet Upstream of					
Cross Section B	0.21	300	458	528	679

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
FOWLER BRANCH					
TRIBUTARY 4					
85 feet Upstream of Cross Section D	0.26	314	402	437	508
185 feet Upstream of Cross Section G	0.20	245	294	312	345
310 feet Upstream of Cross Section G	0.20	372	540	613	771
HARDEE CREEK					
At mouth	0.56	852	1,234	1,399	1,758
At Boulevard Drive	0.30	499	709	800	994
HENDERSON MILL CREEK					
At confluence with Peachtree Branch	1.74	1,695	2,571	2,664	3,837
Downstream of Interstate Route 285	0.63	812	1,248	1,294	1,881
HONEY CREEK					
Below confluence with Honey Creek Tributary A	6.12	1,795	3,315	4,020	5,277
At Rockland Road	4.06	3,852	5,923	6,877	8,950
Below Mall Parkway	1.52	2,156	3,117	3,524	4,373
Below Turner Hill Road	1.46	2,327	3,314	3,732	4,599
At Interstate Route 20	0.29	370	510	567	672
HONEY CREEK					
TRIBUTARY A					
Below Rockland Road	1.52	564	1,085	1,350	1,990
At Forest Lake Parkway	0.77	710	1,207	1,436	1,947

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
IDLE CREEK					
At Cross Section B 315 feet Upstream of Cross Section E	0.89	289	456	491	993
At Cross Section L 40 feet Upstream of Cross Section N	0.69	500	762	911	1,575
1,420 feet Upstream of Cross Section N	0.44	179	429	575	930
2,045 feet Upstream of Cross Section N	0.23	203	442	574	830
	0.23	383	605	704	921
INDIAN CREEK					
At mouth	4.56	3,306	4,564	5,187	7,083
Above confluence with Indian Creek Tributary B	3.92	2,900	3,868	4,556	6,471
Above confluence with Indian Creek Tributary C	3.16	2,489	3,241	3,862	5,195
500 feet upstream of Memorial Drive	2.02	1,687	2,070	2,502	3,423
750 feet downstream of North Decatur Road	0.74	823	1,026	1,100	1,225
At Indian Creek Way	0.33	539	781	885	1,110
INDIAN CREEK TRIBUTARY A					
At mouth	0.76	1,097	1,501	1,597	1,828
750 feet upstream of Northern Avenue	0.51	780	1,124	1,272	1,592
INDIAN CREEK TRIBUTARY B					
At mouth	0.38	384	522	582	682
At Kensington Road	0.26	390	568	645	811
INDIAN CREEK TRIBUTARY C					
At mouth	0.67	971	1,263	1,357	1,537
At Kensington Road	0.49	677	965	1,089	1,355

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
INDIAN CREEK					
TRIBUTARY D					
At mouth	1.23	1,099	1,776	2,066	2,697
520 feet upstream of					
Northern Avenue	0.56	696	1,018	1,157	1,464
At Glendale Road	0.52	662	974	1,110	1,404
INDIAN CREEK					
TRIBUTARY D1					
At mouth	0.24	299	443	505	641
At Hatton Drive	0.20	269	395	450	569
INDIAN CREEK					
TRIBUTARY D2					
At mouth	0.27	228	421	503	675
INTRENCHMENT CREEK					
360 feet Upstream of					
Cross Section C	11.06	4,486	6,587	7,486	9,411
230 feet Upstream of					
Cross Section G	10.81	4,536	6,642	7,540	9,461
160 feet Upstream of					
Cross Section K	10.03	4,833	6,888	7,774	9,714
100 feet Downstream of					
Cross Section M	9.78	4,810	6,850	7,732	9,673
165 feet Upstream of					
Confluence with Tributary 2	9.53	4,780	6,803	7,678	9,613
400 feet Upstream of					
Confluence Tributary 3	8.80	4,694	6,668	7,519	9,437
110 feet Upstream of					
Cross Section W	8.24	4,620	6,565	7,403	9,290
45 feet Downstream of					
Confluence with Tributary A	8.24	4,600	6,537	7,372	9,252
130 feet Upstream of					
Cross Section AB	6.94	4,679	6,620	7,456	9,253

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
INTRENCHMENT CREEK					
TRIBUTARY A					
30 feet Downstream of Eastland Road	1.23	1,300	2,040	2,393	3,146
290 feet Upstream of Cross Section B	0.74	822	1,236	1,412	1,803
820 feet Downstream of Cross Section E	0.69	810	1,206	1,378	1,753
385 feet Upstream of Cross Section I	0.54	713	1,058	1,208	1,534
270 feet Upstream of Cross Section Q	0.21	308	447	507	637
INTRENCHMENT CREEK					
TRIBUTARY 1					
915 feet Upstream of Cross Section A	0.25	301	466	540	700
INTRENCHMENT CREEK					
TRIBUTARY 2					
365 feet Downstream of Cross Section A	0.48	522	791	908	1,166
95 feet Upstream of Cross Section C	0.30	205	307	352	453
130 feet Downstream of Cross Section E	0.18	24	52	66	101
160 feet Upstream of Cross Section G	0.18	290	429	489	619
INTRENCHMENT CREEK					
TRIBUTARY 3					
45 feet Downstream of Cross Section B	0.33	632	895	1,008	1,252
920 feet Upstream of Cross Section B	0.25	495	704	793	987

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
INTRENCHMENT CREEK					
TRIBUTARY 4					
155 feet Upstream of Cross Section D	0.47	614	694	718	742
45 feet Downstream of Ripplewater Drive	0.47	711	1,083	1,246	1,602
280 feet Upstream of Cross Section I	0.30	489	731	837	1,067
JOHNSON CREEK					
At confluence with Yellow River	1.56	511	930	1,246	2,226
At the upstream face of Johnson Creek Drive	1.18	478	864	1,150	2,119
At Johnson Lake Dam	0.76	231	594	803	1,484
LITTLE STONE MOUNTAIN CREEK					
At mouth	2.99	2,135	2,972	3,334	3,909
At dam approximately 200 feet upstream of Stone Mountain Highway	2.65	2,013	2,838	3,134	3,224
At approximately 1,000 feet downstream of Old Stone Mountain Road	2.20	1,976	2,953	3,436	5,157
At Old Stone Mountain Road	2.20	1,870	2,779	3,287	4,918
At approximately 200 feet above Old Stone Mountain Road	1.99	1,748	2,584	3,113	4,625
At confluence with Little Stone Mountain Creek Unnamed Tributary 3	1.73	1,603	2,392	2,940	4,327
Above confluence of Little Stone Mountain Creek Unnamed Tributary 3	1.55	1,443	2,223	2,720	3,967
At Lilburn Stone Mountain Road	1.25	1,184	1,932	2,339	3,345
At confluence with Little Stone Mountain Creek Unnamed Tributary 4	0.90	922	1,546	1,833	2,532
At Gunstock Drive	0.47	552	931	1,106	1,557
At Glacier Drive	0.22	293	482	569	790

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
LULLWATER CREEK					
At confluence with Peavine Creek	2.78	2,192	3,482	3,621	5,391
Downstream of Lower Lullwater Parkway	1.27	1,172	1,922	2,500	3,600
Downstream of Lullwater Parkway	1.25	1,163	1,832	1,904	2,818
Upstream of Lullwater Parkway	1.20	1,100	1,500	1,600	2,500
NANCY CREEK					
At Fulton County boundary	19.67	8,721	13,196	13,674	19,711
Upstream of Perimeter Creek	13.59	6,473	9,856	10,217	14,799
Upstream of Ashford Dunwoody Road	12.18	6,487	9,839	10,198	14,738
Upstream of Seaman Circle	7.49	4,836	7,301	7,504	10,877
Upstream of North Shallowford Road	3.99	2,301	3,460	3,583	5,135
Upstream of Vintage Lane	2.66	1,715	2,609	2,704	3,905

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
NANCY CREEK					
TRIBUTARY A					
At mouth	3.07	1,480	2,027	2,214	2,897
Above Peachford Road	2.57	1,423	2,025	2,227	2,942
At Peeler Road	2.07	1,260	1,789	1,969	2,591
Below Vermack Road	0.64	489	693	762	997
NANCY CREEK					
TRIBUTARY B					
At mouth	1.75	1,197	1,576	1,709	2,267
Upstream of Plantation Lane	1.44	1,079	1,496	1,633	2,179
Upstream of Admiral Drive	1.02	798	1,102	1,203	1,549
NANCY CREEK					
TRIBUTARY C					
At mouth	1.32	1,827	2,529	2,764	3,544
At Donaldson Drive	1.07	1,716	2,332	2,538	3,241
Downstream of Keswick Drive	0.61	1,204	1,595	1,725	2,167
NANCY CREEK					
TRIBUTARY D					
At mouth	1.49	1,223	1,667	1,807	2,298
Below Silver Lake	1.10	1,277	1,786	1,956	2,540
Below Ashford-Dunwoody Road	0.59	736	1,020	1,114	1,438
NANCY CREEK					
TRIBUTARY NO. 1					
At City of Doraville corporate Limits	0.76	570	825	950	1,250
NORTH FORK NANCY CREEK					
At confluence with Nancy Creek	2.48	773	1,206	1,527	3,191
Above Murphy-Candler Lake	1.73	1,335	1,924	2,117	2,787
East of Perimeter Center	0.83	437	704	797	1,129

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
NORTH FORK PEACHTREE CREEK					
At the Fulton County boundary	35.82	11,500	17,699	18,364	26,781
Upstream of Corporate Boulevard	34.14	11,250	17,313	17,965	26,205
Upstream of North Fork Peachtree Creek Tributary A	32.13	10,913	16,848	17,490	25,608
Approximately 400 feet downstream of Interstate Route 85	25.99	9,140	14,034	14,558	21,228
Approximately 900 feet upstream of North Fork Peachtree Creek Tributary D-1	22.41	7,686	11,821	12,265	17,871
Downstream of Shallowford Road	18.97	7,330	11,093	11,495	16,575
Approximately 360 feet downstream of Peachtree Branch	12.17	5,127	7,544	7,800	11,022
Upstream of Chamblee Tucker Road	10.94	5,110	7,497	7,750	10,927
Upstream of Interstate Route 285	9.24	4,077	6,064	6,275	8,933
Upstream of Northcrest Road	7.40	3,996	5,911	6,114	8,667
Upstream of Pleasantdale Road	5.17	3,673	5,375	5,555	7,812
NORTH FORK PEACHTREE CREEK TRIBUTARY A					
At confluence with North Fork Peachtree Creek	3.46	2,225	3,479	3,613	5,321
Upstream of Drew Valley Road	2.35	1,726	2,693	2,797	4,111
Upstream of Dresden Drive	1.24	1,070	1,643	1,705	2,478
NORTH FORK PEACHTREE CREEK TRIBUTARY B					
At confluence with North Fork Peachtree Creek	3.14	2,294	3,451	3,576	5,141
Downstream of Plaster Road	2.38	2,158	3,177	3,285	4,633
Downstream of Buford Highway	1.43	1,447	2,130	2,202	3,106

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
NORTH FORK PEACHTREE CREEK TRIBUTARY C					
At confluence with North Fork Peachtree Creek	1.03	948	1,503	1,563	2,323
Downstream of Archwood Drive	0.59	657	1,035	1,075	1,590
NORTH FORK PEACHTREE CREEK TRIBUTARY D-1					
At confluence with North Fork Peachtree Creek	2.53	1,249	2,131	2,227	3,421
Upstream of Echo Drive	2.29	1,492	2,416	2,515	3,775
Upstream of Circlewood Drive	0.97	606	1,024	1,069	1,642
NORTH FORK PEACHTREE CREEK TRIBUTARY D-2					
At confluence with North Fork Peachtree Creek Tributary D-1	1.03	942	1,506	1,567	2,342
Upstream of Deroran Drive	0.6	587	939	977	1,427
NORTH FORK PEACHTREE CREEK TRIBUTARY D-3					
At confluence with North Fork Peachtree Creek Tributary D-2	0.50	554	890	926	1,389
Upstream of Gwinevere Drive	0.24	316	514	534	810
PANTHERS BRANCH					
1,660 feet upstream of Confluence with Snapfinger Creek	2.60	2,930	3,827	4,217	5,094
Downstream of confluence of Panthers Branch Tributary 1	2.43	2,922	3,771	4,134	4,993
1,300 feet upstream of Zinzendorf Drive	1.94	2,458	3,065	3,340	3,899
150 feet downstream of Rock Springs Road	1.67	2,173	2,680	2,862	3,252
25 feet upstream of Rock Springs Road	1.42	1,798	2,206	2,331	2,653

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
PANTHERS BRANCH					
(continued)					
600 feet downstream of Thompson Mill Road	1.26	1,641	2,225	2,377	2,710
20 feet upstream of Thompson Mill Road	1.00	1,279	1,656	1,749	1,938
1,000 feet upstream of Thompson Mill Road	1.00	1,426	1,807	1,965	2,293
150 feet upstream of Minola Drive	0.82	1,153	1,404	1,502	1,696
75 feet upstream of Interstate-20	0.70	968	1,155	1,223	1,351
600 feet downstream of Easterly Place	0.63	1,050	1,334	1,454	1,712
600 feet upstream of Easterly Place	0.49	776	951	1,025	1,186
50 feet downstream of Snapfinger Woods Drive	0.38	558	663	702	781
40 feet upstream of Snapfinger Woods Drive	0.32	456	531	557	605
30 feet downstream of Dividend Drive	0.24	485	656	729	886
PANTHERS BRANCH					
TRIBUTARY 1					
430 feet upstream of Confluence with Panthers Branch	0.27	380	600	751	1,025
At Cross Section B, northeast of Earlhan Court	0.26	364	576	722	976
250 feet upstream of Cross Section C	0.20	361	528	603	768
PANTHERS BRANCH					
TRIBUTARY 2					
450 feet upstream of Confluence with Panthers Branch	0.25	396	557	667	896
800 feet upstream of Rock Springs Road	0.23	394	581	665	850

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
PANTHERS BRANCH					
TRIBUTARY 3					
800 feet upstream of confluence with Panthers Branch	0.22	385	550	622	780
PEACHTREE BRANCH					
At confluence with North Fork Peachtree Creek	5.44	3,164	4,942	5,136	6,481
Upstream of Bolero Drive	3.36	2,233	3,557	3,700	5,520
Upstream of Evans Dale Circle	1.92	1,516	2,403	2,499	3,714
Downstream of Lone Star Trail	1.37	1,289	2,042	2,123	3,154
PEAVINE CREEK					
At confluence with South Fork Peachtree Creek	5.94	3,556	5,576	5,794	8,557
Just upstream of Clifton Road	2.16	1,136	1,437	1,539	1,845
Just upstream of CSX railroad	0.66	441	919	1,157	2,085
Just downstream of Coventry Road	0.62	426*	911*	1,147*	2,050*
Just downstream of Scott Boulevard	0.42	971	1,375	1,550	2,170
PEAVINE CREEK LAMONT TRIBUTARY					
At mouth	0.17	397	500	572	993
PEAVINE CREEK TRIBUTARY					
Just downstream of Westchester Drive	0.93	1,137	1,423	1,553	2,156
At Garden Lane	0.79	1,050	1,279	1,399	1,981
Just upstream of Lamont Drive	0.72	987	1,164	1,285	1,897
At West Ponce De Leon Ave	0.47	638	728	755	838
PEAVINE CREEK UPLAND TRIBUTARY					
Approximately 100 feet upstream of Upland Road	0.14	326	461	518	709

*Discharge decreases due to storage routing upstream of Coventry Road.

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
PERIMETER CREEK					
At confluence with Nancy Creek	3.30	2,452	3,193	3,489	4,596
Upstream of Byrnwyck Road	2.76	2,496	3,244	3,513	4,714
Upstream of Hammond Drive	1.09	828	1,172	1,329	1,576
PINE MOUNTAIN CREEK					
At confluence with Tom George Creek	1.74	786	1,095	1,221	1,593
At dirt road	1.26	688	913	1,000	1,225
Above confluence with Pine Mountain Creek Unnamed Tributary 1	0.94	828	1,319	1,579	2,209
At Turner Hill Road	0.89	807	1,289	1,541	2,148
PLUNKETT CREEK					
At mouth	1.70	1,174	2,005	2,381	3,219
Upstream of Plunkett Creek Unnamed Tributary	0.64	498	801	952	1,290
At Rockland Road	0.21	174	292	347	468
PLUNKETT CREEK UNNAMED TRIBUTARY					
At mouth	0.58	548	950	1,137	1,536
At dam	0.32	404	663	777	1,015

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
POLE BRIDGE CREEK					
At mouth	18.29	4,366	7,168	8,421	11,068
At Confluence of Pole					
Bridge Creek Tributary C	17.96	4,404	7,282	8,551	11,230
Above Browns Mill Road	17.53	4,409	7,306	8,577	11,261
At Confluence of Pole					
Bridge Creek Tributary D	16.71	4,458	7,456	8,730	11,457
At Confluence of Pole					
Bridge Creek Tributary Q	15.72	4,410	7,389	8,652	11,363
At Confluence of Pole					
Bridge Creek Tributary E	14.83	4,345	7,290	8,535	11,204
At Confluence of Pole					
Bridge Creek Tributary F	14.40	4,395	7,402	8,643	11,418
At Rock Spring Road	13.06	4,254	7,189	8,399	11,098
At Confluence of Pole					
Bridge Creek Tributary A	12.50	4,372	7,373	8,564	11,799
At Confluence of Pole					
Bridge Creek Tributary G	10.27	5,212	7,521	8,574	10,957
At Confluence of Pole					
Bridge Creek Tributary P	9.23	5,290	7,556	8,598	10,968
At Confluence of Pole					
Bridge Creek Tributary H	8.87	5,293	7,505	8,527	10,879
Above Interstate Route 20	7.80	4,904	6,877	7,734	9,674

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
POLE BRIDGE CREEK					
(continued)					
At Confluence of Pole					
Bridge Creek Tributary B	7.54	4,949	6,910	7,753	9,650
At Covington Highway	3.75	2,591	3,708	4,201	5,400
At Confluence of Pole					
Bridge Creek Tributary J	3.61	2,590	3,704	4,190	5,366
At Confluence of Pole					
Bridge Creek Tributary K	3.20	2,656	3,775	4,259	5,358
At Confluence of Pole					
Bridge Creek Tributary L	2.61	2,539	3,573	4,008	4,953
At Confluence of Pole					
Bridge Creek Tributary M	1.40	1,495	2,213	2,594	3,478
At Confluence of Pole					
Bridge Creek Tributary N	0.97	1,118	1,695	2,062	2,817
At Confluence of Pole					
Bridge Creek Tributary O	0.39	393	693	837	1,213
POLE BRIDGE CREEK					
TRIBUTARY A					
At mouth	2.16	1,445	1,924	2,139	2,578
Below Fairington Road	1.94	1,296	1,532	1,629	1,856
Above Interstate Route 20	1.55	2,201	3,039	3,386	4,109
Above Snapfinger Woods Drive	0.74	1,181	1,744	2,034	2,555
At dam	0.23	533	775	859	986
POLE BRIDGE CREEK					
TRIBUTARY AA					
At Keystone Gates Drive	0.67	1,210	1,730	1,961	2,469
Below Concepts 21 Drive	0.19	385	542	610	756
POLE BRIDGE CREEK					
TRIBUTARY AAA					
Below Snapfinger Woods Drive	0.48	884	1,286	1,465	1,856
Above Shirewick Lane	0.27	590	847	960	1,207

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
POLE BRIDGE CREEK					
TRIBUTARY B					
At mouth	3.33	2,562	3,579	4,083	5,200
At Covington Highway	2.78	2,423	3,089	3,480	4,271
At Confluence of Pole					
Bridge Creek Tributary BA	2.69	2,429	3,106	3,457	4,226
Above Marbut Road	1.64	1,811	2,151	2,276	2,494
At Phillips Road	0.46	587	927	1,071	1,227
POLE BRIDGE CREEK					
TRIBUTARY BA					
Below Lithonia Industrial					
Parkway	0.59	1,558	2,147	2,405	2,956
Above Lithonia Industrial					
Parkway	0.50	1,317	1,814	2,032	2,497
Above Pole Bridge Creek					
Tributary BAA	0.27	783	1,068	1,190	1,453
POLE BRIDGE CREEK					
TRIBUTARY BAA					
Below Tribble Street	0.22	513	725	817	1,016
POLE BRIDGE CREEK					
TRIBUTARY BB					
At mouth	0.14	485	658	732	891
POLE BRIDGE CREEK					
TRIBUTARY BC					
At mouth	0.90	1,607	2,080	2,361	2,953
At dam near Phillips Place	0.22	357	511	578	725
POLE BRIDGE CREEK					
TRIBUTARY BD					
Above Lithonia Industrial					
Boulevard	0.40	800	1,152	1,308	1,640
Below Covington Highway	0.20	513	735	832	1,044
POLE BRIDGE CREEK					
TRIBUTARY C					
At mouth	0.24	245	366	420	541

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
POLE BRIDGE CREEK					
TRIBUTARY D					
At mouth	0.42	717	1,052	1,201	1,530
Near Salem Mill Terrace	0.14	278	409	468	598
POLE BRIDGE CREEK					
TRIBUTARY E					
Below Hunters Hill Drive	0.35	618	898	1,023	1,296
Above Hunters Hill Drive	0.22	412	597	679	858
POLE BRIDGE CREEK					
TRIBUTARY F					
At mouth	1.33	2,266	3,306	3,797	5,033
At Confluence of Pole Bridge Creek Tributary FA	1.04	1,802	2,636	3,027	4,056
At Confluence of Pole Bridge Creek Tributary FB	0.57	731	1,171	1,399	2,094
Below Brisbane Way	0.34	585	956	1,139	1,512
Above Timor Trail	0.21	475	683	774	973
POLE BRIDGE CREEK					
TRIBUTARY FA					
At mouth	0.46	1,148	1,645	1,861	2,332
Near Hampton Club Way	0.17	432	623	707	891
POLE BRIDGE CREEK					
TRIBUTARY FB					
At mouth	0.23	196	286	326	595
At Winslow Crossing	0.16	45	70	134	365
POLE BRIDGE CREEK					
TRIBUTARY G					
At mouth	0.67	248	360	410	520
At Evans Mill Road	0.21	436	639	729	927
POLE BRIDGE CREEK					
TRIBUTARY H					
At Lithonia Industrial Boulevard	1.06	1,304	2,118	2,474	3,252
Above Evans Mill Road	0.21	382	559	637	811

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
POLE BRIDGE CREEK					
TRIBUTARY HA					
At mouth	0.24	413	690	813	1,069
Above Glen Road	0.19	431	627	714	906
POLE BRIDGE CREEK					
TRIBUTARY J					
At mouth	0.28	446	663	759	974
Near Marbut Farms Lane	0.19	384	555	630	795
POLE BRIDGE CREEK					
TRIBUTARY K					
At mouth	0.26	532	769	874	1,103
At Marbut Road	0.19	407	588	668	843
POLE BRIDGE CREEK					
TRIBUTARY L					
At mouth	0.99	1,786	2,616	2,976	3,769
Above Wellborn Oaks Court	0.95	1,898	2,746	3,124	3,957
At Marbut Road	0.24	543	787	895	1,131
POLE BRIDGE CREEK					
TRIBUTARY LA					
Below Wellborn Creek Court	0.27	568	807	911	1,138
At Briar Knoll Road	0.18	376	532	600	747
POLE BRIDGE CREEK					
TRIBUTARY M					
At mouth	0.35	682	982	1,115	1,404
At dam	0.24	448	646	732	922
POLE BRIDGE CREEK					
TRIBUTARY N					
At mouth	0.35	735	1,045	1,180	1,473
At Young Road	0.29	620	876	988	1,230
POLE BRIDGE CREEK					
TRIBUTARY O					
Below Stoneleigh Hill Road	0.21	295	490	595	874

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
POLE BRIDGE CREEK					
TRIBUTARY P					
Below Interstate Route 20	0.20	342	571	602	619
Above Interstate Route 20	0.20	464	654	736	914
POLE BRIDGE CREEK					
TRIBUTARY Q					
At mouth	0.75	755	1,170	1,356	1,766
Below Lions Gate Court	0.40	489	787	919	1,209
SHOAL CREEK					
At Flat Shoals Drive	19.41	5,215	7,644	10,244	11,331
1,000 feet Downstream of					
Cross Section B	18.48	5,663	8,408	11,007	11,999
At Columbia Drive	8.62	3,954	4,759	5,059	5,669
At Cross Section E	8.62	4,069	5,045	5,484	6,535
Upstream of Interstate 285	8.33	3,995	4,886	5,252	6,174
Downstream of Confluence					
with Tributary 1	8.02	3,948	4,796	5,092	5,713
800 feet Upstream of					
Cross Section G	7.73	3,871	4,707	4,971	5,480
At Cross Section H	7.57	3,850	4,661	4,922	5,423
At Rainbow Drive	7.40	3,830	4,616	4,874	5,367
1,000 feet Upstream of					
Rainbow Drive	7.40	3,831	4,618	4,879	5,380
500 feet Upstream of					
Ember Drive	7.28	3,814	4,576	4,834	5,326
At Interstate 20	7.11	3,789	4,517	4,770	5,286
Downstream of Confluence					
with Tributary A	7.11	4,282	6,170	6,998	8,813
Downstream of Confluence					
with Tributary 2	6.11	3,697	4,282	4,887	6,120
400 feet Downstream of					
Toney Drive	5.89	3,664	4,201	4,582	5,577
500 feet Upstream of					
Toney Drive	5.81	3,655	4,189	4,526	5,479
Downstream of Confluence					
with Tributary 3	5.60	3,625	4,153	4,356	5,050
Downstream of Confluence					
with Tributary 4	5.41	3,596	4,120	4,320	5,006

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SHOAL CREEK (continued)					
At Cross Section P	5.34	3,588	4,110	4,310	4,993
At McAfee Road (Cross Section P)	5.01	3,534	4,049	4,246	4,915
1,100 feet Upstream of McAfee Road	5.01	3,886	4,760	5,159	6,549
At Cross Section R	4.80	3,854	4,716	5,111	6,487
At Glenwood Road	4.29	3,755	4,577	4,962	6,299
800 feet Upstream of Glenwood Road	4.29	4,100	5,393	5,931	7,012
At White Oak Drive (Cross Section T)	4.16	4,091	5,366	5,904	6,966
Downstream of Confluence with Tributary 7	4.14	4,224	5,497	6,038	7,184
2,000 feet Upstream of White Oak Drive	3.77	3,856	4,964	5,422	6,354
Downstream of Confluence with Tributary 8	3.55	3,740	4,797	5,245	6,172
Downstream of Line Street	3.36	3,612	4,647	5,101	6,020
Downstream of Confluence with Tributary 9	3.31	3,604	4,633	5,084	6,011
At Memorial Drive	3.14	3,528	4,546	5,000	5,914
300 feet Upstream of Memorial Drive	3.14	3,606	4,897	5,546	7,529
1,000 feet Upstream of Confluence with Shoal Creek					
East Fork Middle Branch	1.81	1,964	2,769	3,136	4,394
At Midway Road	1.48	1,805	2,468	2,835	4,782
Just downstream of Kirk Road	0.53	351	637	857	1,624
Just downstream of Hilldale Drive	0.23	636	870	970	1,298
Just downstream of South Columbia Drive	0.17	535	722	801	1,062
SHOAL CREEK DRIFTWOOD TRIBUTARY					
At mouth	0.21	399	639	734	1,028
Just downstream of Garland Avenue	0.17	331	523	599	835

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SHOAL CREEK EAST FORK MIDDLE BRANCH					
200 feet Upstream of Confluence with Shoal Creek	1.24	1,684	2,164	2,369	3,255
600 feet Upstream of Cross Section A	1.20	1,651	2,109	2,335	3,217
500 feet Downstream of Columbia Drive	1.01	1,401	1,768	2,048	2,835
At Columbia Drive	0.84	1,267	1,600	1,864	2,586
500 feet Downstream of Cross Section F	0.84	1,481	2,167	2,485	3,159
Downstream of Confluence with East Fork Middle Branch Tributary 1	0.59	1,156	1,665	1,892	2,397
600 feet Upstream of Cross Section G	0.23	555	793	897	1,124
At Cross Section H	0.13	289	419	476	601
SHOAL CREEK EAST FORK MIDDLE BRANCH TRIBUTARY 1					
350 feet Upstream of Katie Kerr Drive	0.26	409	599	683	885
SHOAL CREEK KIRK TRIBUTARY					
At mouth	0.10	192	281	319	446
SHOAL CREEK TRIBUTARY A					
600 feet Downstream of Glendale Drive	0.94	1,431	1,902	2,101	2,665
800 feet Upstream of Glendale Drive	0.91	1,408	1,864	2,057	2,634
200 feet Upstream of Toney Drive	0.78	1,205	1,561	1,746	2,368
700 feet Upstream of Toney Drive	0.76	1,200	1,548	1,735	2,369
750 feet Upstream of Shamrock Drive	0.60	954	1,209	1,454	1,984

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SHOAL CREEK					
TRIBUTARY A (continued)					
At Irish Lane					
(Cross Section H)	0.48	771	1,006	1,204	1,636
200 feet Upstream of Irish Lane	0.48	935	1,322	1,493	1,866
200 feet Upstream of McAfee Road	0.33	621	883	998	1,249
At Rosewood Rd					
(Cross Section L)	0.24	454	645	728	909
200 feet Upstream of Rosewood Road	0.21	417	591	667	831
SHOAL CREEK					
TRIBUTARY 1					
150 feet Upstream of Shoal Creek Road	0.25	108	136	147	236
Dam Downstream DaVinci Blvd. (Cross Section C)	0.22	70	72	107	224
Upstream end of pond below DaVinci Blvd	0.22	425	613	697	879
At Cross Section E	0.15	304	435	493	618
SHOAL CREEK					
TRIBUTARY 2					
At Cross Section B	0.22	564	797	899	1,119
SHOAL CREEK					
TRIBUTARY 3					
At Cross Section C	0.19	421	607	689	868
SHOAL CREEK					
TRIBUTARY 4					
At Cross Section C	0.21	360	522	593	750
SHOAL CREEK					
TRIBUTARY 5					
800 feet Upstream of Cross Section D	0.27	509	733	831	1,046

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SHOAL CREEK					
TRIBUTARY 6					
At Santa Cruz Drive	0.21	271	376	470	629
At Cross Section E	0.21	311	449	510	643
SHOAL CREEK					
TRIBUTARY 7					
300 feet Downstream of Cross Section A	0.37	639	947	1,077	1,353
At Santa Monica Drive (Cross Section B)	0.35	630	931	1,059	1,329
Between Monterey Drive and Santa Monica Drive	0.35	696	975	1,096	1,358
At Cross Section E	0.20	438	603	674	827
Upstream of Catalina Drive	0.17	383	525	585	716
SHOAL CREEK					
TRIBUTARY 8					
At Cross Section B	0.15	248	359	409	517
SHOAL CREEK					
TRIBUTARY 9					
At Cross Section C	0.15	325	471	535	676
SHOAL CREEK WEST					
TRIBUTARY					
Just downstream of South Candler Street	0.72	1,216	1,665	1,839	2,816
Just downstream of Buchanan Terrace	0.51	820	1,159	1,368	2,159
At West Benson Street	0.41	667	1,098	1,302	1,973
Just downstream of South McDonough Street	0.24	397	697	817	1,179
Just upstream of Adams Street	0.22	495	718	813	1,129
Just downstream of Ansley Street	0.13	338	478	538	735

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
1400 feet upstream of the confluence with South River	38.49	10,237	14,359	16,383	20,089
1180 feet upstream of Dogwood Farm Road	38.30	10,247	14,368	16,398	20,107
350 feet upstream of confluence of Snapfinger Creek Tributary 1	37.94	10,257	14,373	16,409	20,122
120 feet upstream of confluence of Snapfinger Creek Tributary 2	37.34	10,244	14,343	16,379	20,082
50 feet downstream of confluence of Panthers Branch	36.98	10,337	14,452	16,535	20,279
930 feet upstream of confluence of Panthers Branch	34.37	10,127	14,152	16,190	19,838
75 feet downstream of Snapfinger Road	34.21	10,148	14,170	16,217	19,872
450 feet upstream of Cross Section K	33.02	10,062	14,044	16,071	19,682
230 feet downstream of Thompson Mill Road	32.58	10,107	14,088	16,134	19,765
980 feet upstream of Thompson Mill Road	32.43	10,097	14,074	16,118	19,744
150 feet upstream of confluence of Snapfinger Creek Tributary 3	32.30	10,093	14,066	16,109	19,732
550 feet downstream of Interstate 20	31.10	9,971	13,912	15,950	19,558
200 feet upstream of Interstate 20	30.94	9,959	13,894	15,930	19,532
30 feet downstream of confluence of Snapfinger Creek Tributary 4	30.71	10,007	13,948	15,999	19,620
200 feet upstream of Snapfinger Woods Drive	30.18	9,991	13,921	15,972	19,587
270 feet upstream of confluence of Snapfinger Creek Tributary 5	29.57	10,048	14,012	16,082	19,733

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK (continued)					
150 feet upstream of confluence of Snapfinger Creek Tributary 6	28.81	10,064	14,045	16,129	19,798
140 feet upstream of confluence of Snapfinger Creek Tributary 7	28.33	10,048	14,033	16,109	19,786
120 feet upstream of confluence of Snapfinger Creek Tributary 8	28.03	10,044	14,048	16,124	19,807
At confluence of Snapfinger Creek Tributary 9	27.69	10,039	14,050	16,086	19,728
700 feet downstream of South Hairston Road	26.14	10,476	14,606	16,718	20,452
100 feet upstream of South Hairston Road	17.09	7,991	10,305	11,615	14,787
100 feet upstream of confluence of Snapfinger Creek Tributary 10	16.94	9,511	13,179	14,992	19,031
320 feet upstream of confluence of Snapfinger Creek Tributary 11	16.23	9,424	13,048	14,841	18,836
250 feet downstream of Cross Section AP	15.71	9,474	13,123	14,949	19,022
50 feet upstream of confluence of Snapfinger Creek Tributary 12	15.55	9,479	13,128	14,961	19,049
400 feet upstream of confluence of Snapfinger Creek Tributary B	14.84	9,562	13,255	15,190	19,450
100 feet downstream of confluence of Snapfinger Creek Tributary 13	13.41	9,457	13,049	15,001	19,218
800 feet upstream of Redan Road	13.02	9,507	13,119	14,959	19,141
280 feet upstream of Rowland Road	12.90	9,664	13,375	15,301	19,654
330 feet downstream of Cross Section BB	8.34	6,529	9,068	10,187	12,593

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
(continued)					
250 feet upstream of confluence of Snapfinger Creek Tributary 14	8.08	6,571	9,139	10,273	12,703
1,050 feet downstream of Rockbridge Road	7.13	6,466	8,976	10,081	12,432
820 feet upstream of Rockbridge Road	6.88	6,533	9,062	10,175	12,533
130 feet upstream of confluence of Snapfinger Creek Tributary C	6.76	6,655	9,336	10,506	12,996
50 feet upstream of Rays Road	4.58	4,233	5,567	6,118	7,280
At Cross Section BM	4.38	4,207	5,500	6,033	7,148
125 feet upstream of confluence of Snapfinger Creek Tributary D	4.26	4,151	5,409	5,919	6,970
90 feet upstream of Cross Section BP	3.27	3,241	4,113	4,445	5,161
50 feet upstream of Park Drive	3.09	3,160	4,003	4,312	4,946
40 feet upstream of Spruce Drive	2.95	3,606	4,461	4,806	5,522
1,100 feet upstream of Spruce Drive	2.78	3,467	4,238	4,538	5,153
100 feet upstream of Memorial Drive	2.30	3,032	3,685	3,915	4,379
500 feet downstream of Anderson Road	2.27	3,470	4,296	4,612	5,278
75 feet downstream of confluence of Snapfinger Creek Tributary E	1.93	2,991	3,591	3,824	4,272
At Cross Section CA	1.14	1,586	1,928	2,041	2,302
115 feet upstream of North Hairston Road	1.03	1,485	1,831	1,943	2,176
150 feet upstream of Cross Section CD	0.89	1,317	1,726	1,850	2,221
475 feet upstream of Central Drive	0.80	1,260	1,679	1,815	2,080
40 feet upstream of Sheila Lane	0.59	921	1,248	1,338	1,495

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK (continued)					
250 feet downstream of confluence of Snapfinger Creek Tributary 17	0.40	660	923	1,035	1,278
350 feet upstream of confluence of Snapfinger Creek Tributary 17	0.22	406	549	611	743
SNAPFINGER CREEK TRIBUTARY A					
60 feet upstream of Flat Shoals Parkway	1.19	1,143	1,522	1,668	2,346
250 feet downstream of confluence of Snapfinger Creek Tributary A1	0.91	840	1,193	1,378	2,065
25 feet upstream of Wesley Bend	0.58	593	791	895	1,290
1,300 feet upstream of Harvest Hill Court	0.48	518	753	857	1,315
620 feet downstream of Cross Section G	0.36	274	448	640	1,062
At dam of pond 1400 feet downstream of Autumn Lake Lane	0.22	81	308	431	702
40 feet downstream of Autumn Lake Lane	0.22	425	606	685	857
SNAPFINGER CREEK TRIBUTARY A1					
30 feet upstream of Snapfinger Lake Drive	0.33	384	479	538	835
890 feet upstream of Sterling Forest Drive	0.18	294	400	440	615
225 feet upstream of Cross Section E	0.18	309	442	499	626

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPPFINGER CREEK					
TRIBUTARY B					
870 feet upstream of confluence with Snapfinger Creek	1.08	1,362	1,994	2,257	2,870
50 feet downstream of South Indian Creek Drive	0.97	1,306	1,919	2,175	2,763
820 feet upstream of South Indian Creek Drive	0.75	1,100	1,586	1,804	2,277
50 feet downstream of confluence of Snapfinger Creek Tributary B1	0.53	1,148	1,605	1,807	2,241
625 feet downstream of June Drive	0.26	704	964	1,073	1,315
125 feet upstream of June Drive	0.22	636	865	963	1,175
50 feet downstream of Interstate 285	0.16	489	655	726	879
SNAPPFINGER CREEK					
TRIBUTARY B1					
870 feet upstream of confluence with Snapfinger Creek Tributary B	0.27	470	676	767	964
200 feet downstream of Jane Marie Lane	0.08	204	286	321	398

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY C					
1,090 feet upstream of confluence with Snapfinger Creek	2.14	2,982	4,182	4,700	5,873
25 feet downstream of Memorial Drive	1.96	2,870	3,998	4,483	5,606
215 feet upstream of Memorial Drive	1.90	2,819	3,927	4,404	5,511
660 feet upstream of Memorial Drive	1.79	2,685	3,736	4,194	5,258
20 feet upstream of Memorial College Avenue	1.71	2,672	3,684	4,120	5,183
1,130 feet upstream of Memorial College Avenue	1.55	2,386	3,278	3,684	4,664
100 feet downstream of confluence of Snapfinger Creek Tributary C1	1.30	2,050	2,724	3,105	3,976
At Cross Section F	0.89	1,518	2,021	2,318	2,973
Downstream of Milam Park	0.60	1,039	1,488	1,687	2,125
Between Norman Road and Milam Park	0.45	783	1,120	1,270	1,601
645 feet upstream of Norman Road	0.45	129	141	144	146
890 feet upstream of Norman Road	0.45	143	163	180	186
985 feet upstream of Norman Road	0.45	312	343	348	352
1,035 feet upstream of Norman Road	0.45	588	728	870	938
1,055 feet upstream of Norman Road	0.45	770	1,087	1,188	1,462
925 feet downstream of Reilly Lane	0.45	783	1,120	1,270	1,601
330 feet upstream of Reilly Lane	0.18	301	442	506	646

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY C SPLIT					
625 feet upstream of Norman Road	*	657	980	1,126	1,455
830 feet upstream of Norman Road	*	643	958	1,091	1,415
865 feet upstream of Norman Road	*	475	777	922	1,248
900 feet upstream of Norman Road	*	198	393	400	663
950 feet upstream of Norman Road	*	17	33	82	139
SNAPFINGER CREEK					
TRIBUTARY C1					
500 feet upstream of Holly Hedge Road	0.41	539	720	808	1,005
850 feet downstream of Norman Road	0.34	437	584	669	873
30 feet upstream of Norman Road	0.21	279	414	524	716
250 feet upstream of Norman Road	0.21	349	511	582	741
SNAPFINGER CREEK					
TRIBUTARY D					
1,200 feet upstream of confluence with Snapfinger Creek	0.97	1,102	1,489	1,650	1,980
25 feet downstream of Memorial Drive	0.83	882	1,180	1,305	1,566
40 feet upstream of Memorial Drive	0.80	855	1,138	1,249	1,472
185 feet upstream of Village Square Court	0.68	763	1,008	1,100	1,241
At Cross Section D	0.65	737	973	1,061	1,196
475 feet upstream of Abingdon Drive	0.56	823	1,150	1,277	1,684
950 feet upstream of Abingdon Drive	0.40	581	770	856	1,245

*Data not available

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY D (continued)					
550 feet downstream of Central Drive	0.35	504	652	745	1,093
50 feet upstream of Central Drive	0.29	447	579	671	975
100 feet downstream of Forest Heights Drive	0.29	464	677	772	981
SNAPFINGER CREEK					
TRIBUTARY E					
1,170 feet upstream of confluence with Snapfinger Creek	0.80	1,426	1,719	1,827	2,044
25 feet upstream of Central Drive	0.69	1,272	1,513	1,612	1,778
250 feet downstream of Cherie Glen Trail	0.53	1,195	1,671	1,877	2,324
115 feet upstream of Cherie Glen Trail	0.34	913	1,261	1,411	1,734
925 feet upstream of Cedar Park Drive	0.32	874	1,204	1,345	1,650
SNAPFINGER CREEK					
TRIBUTARY E1					
1,170 feet upstream of confluence with Snapfinger Creek Tributary E	0.18	290	421	479	606
185 feet upstream of Cherie Glen Trail	0.17	274	398	452	572

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY 1					
740 feet upstream of confluence with Snapfinger Creek	0.54	628	979	1,146	1,553
150 feet upstream of Dogwood Farms Drive	0.42	472	740	873	1,202
800 feet upstream of Dogwood Farms Drive	0.37	438	682	802	1,092
450 feet downstream of Tarragon Drive	0.22	326	474	540	684
275 feet upstream of Tarragon Drive	0.18	278	402	457	577
SNAPFINGER CREEK					
TRIBUTARY 2					
1,130 feet upstream of confluence with Snapfinger Creek	0.22	434	634	723	918
SNAPFINGER CREEK					
TRIBUTARY 3					
250 feet upstream of confluence with Snapfinger Creek	1.02	768	1,252	1,408	1,572
150 feet upstream of Minola Drive	0.90	723	1,210	1,363	1,521
100 feet upstream of Interstate 20	0.80	693	1,169	1,321	1,475
275 feet upstream of Interstate 20	0.80	697	1,244	1,380	1,533
40 feet upstream of Panola Industrial Boulevard	0.61	1,103	1,452	1,579	1,803
50 feet upstream of Snapfinger Woods Drive	0.49	982	1,377	1,549	1,925
625 feet upstream of Snapfinger Woods Drive	0.46	924	1,297	1,461	1,814
1,900 feet upstream of Snapfinger Woods Drive	0.16	377	526	590	730

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY 4					
785 feet upstream of confluence with Snapfinger Creek	0.50	795	1,077	1,188	1,409
50 feet downstream of Snapfinger Woods Drive	0.48	802	1,074	1,183	1,399
75 feet upstream of Rayburn Road	0.28	549	687	732	816
50 feet upstream of Interstate 20	0.15	218	241	248	259
800 feet upstream of Interstate 20	0.15	313	444	501	626
SNAPFINGER CREEK					
TRIBUTARY 5					
360 feet upstream of confluence with Snapfinger Creek	0.76	905	1,179	1,281	1,506
400 feet downstream of South Hairston Road	0.72	857	1,108	1,205	1,409
40 feet upstream of South Hairston Road	0.53	556	692	741	838
175 feet upstream of Private Driveway at end of Langdon Court	0.33	534	779	886	1,121
275 feet upstream of Wesley Meadows Drive	0.24	407	590	671	849
215 feet upstream of Cross Lane	0.15	262	378	429	541
SNAPFINGER CREEK					
TRIBUTARY 6					
680 feet upstream of confluence with Snapfinger Creek	0.28	295	464	521	598
50 feet upstream of Marsh Rabbit Lane	0.24	267	417	468	527
110 feet upstream of South Hairston Road	0.17	319	463	527	667

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY 7					
700 feet upstream of confluence with Snapfinger Creek	0.16	179	324	389	516
75 feet upstream of Creekside Place	0.14	171	308	367	485
200 feet upstream of Creekside Place	0.14	257	364	410	511
SNAPFINGER CREEK					
TRIBUTARY 8					
720 feet upstream of confluence with Snapfinger Creek	0.22	215	282	311	373
At detention pond 250 feet downstream of Red Branch Drive	0.15	117	138	146	250
75 feet downstream of Red Branch Drive	0.15	280	401	454	569
SNAPFINGER CREEK					
TRIBUTARY 9					
790 feet upstream of confluence with Snapfinger Creek	1.39	1,885	2,714	3,078	3,883
410 feet downstream of Biffle Road	1.39	1,959	2,808	3,187	4,023
165 feet upstream of Biffle Road	1.14	1,592	2,293	2,606	3,294
410 feet upstream of Cross Section C	0.76	1,098	1,603	1,831	2,336
800 feet downstream of South Hidden Hills Parkway	0.67	964	1,413	1,614	2,061
475 feet downstream of South Hidden Hills Parkway	0.53	756	1,104	1,260	1,607
575 feet upstream of South Hidden Hills Parkway	0.40	586	861	984	1,255
215 feet upstream of Cross Section I	0.20	292	428	488	621

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY 9A					
330 feet upstream of Biffle Road	0.23	342	498	568	719
225 feet upstream of Young Road	0.20	305	444	505	639
SNAPFINGER CREEK					
TRIBUTARY 9B					
50 feet upstream of South Hidden Hills Parkway	0.36	488	681	763	943
510 feet upstream of Young Road	0.32	437	608	681	840
50 feet upstream of Greens Brooke Close	0.21	291	411	463	576
575 feet upstream of Greens Brooke Close	0.21	308	452	515	656
SNAPFINGER CREEK					
TRIBUTARY 10					
260 feet upstream of confluence with Snapfinger Creek	0.27	542	772	873	1,094
At Cross Section B	0.21	417	597	676	848
SNAPFINGER CREEK					
TRIBUTARY 11					
330 feet upstream of Denise Court	0.37	699	1,012	1,150	1,453
400 feet upstream of Glenhaven Drive	0.22	482	692	785	987

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY 12					
490 feet upstream of confluence with Snapfinger Creek	0.52	576	824	903	1,177
65 feet upstream of Allgood Road	0.48	562	805	882	1,151
120 feet upstream of Cross Section B	0.40	482	801	934	1,224
525 feet downstream of To Lani Drive	0.28	457	665	757	959
150 feet downstream of To Lani Drive	0.18	285	417	475	603
SNAPFINGER CREEK					
TRIBUTARY 13					
30 feet downstream of Redan Road	0.30	322	379	397	573
30 feet upstream of Redan Road	0.30	321	377	395	570
330 feet upstream of Lake Breeze Drive	0.21	338	501	573	734
SNAPFINGER CREEK					
TRIBUTARY 14					
1,370 feet upstream of confluence with Snapfinger Creek	0.42	719	1,048	1,194	1,515
75 feet downstream of Allgood Road	0.30	596	869	989	1,255
370 feet upstream of Allgood Road	0.21	408	594	676	857
635 feet upstream of Allgood Road	0.14	286	417	476	604

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SNAPFINGER CREEK					
TRIBUTARY 15					
25 feet upstream of Rue Montaigne	0.33	572	835	949	1,200
130 feet upstream of South Rays Road	0.32	561	817	929	1,174
215 downstream of Cross Section E	0.25	438	635	724	913
165 feet downstream of Woodcrest Manor Drive	0.16	274	398	453	573
SNAPFINGER CREEK					
TRIBUTARY 16					
975 feet upstream of confluence with Snapfinger Creek	0.17	287	416	473	598
SNAPFINGER CREEK					
TRIBUTARY 17					
At dam of pond 100 feet upstream of confluence with Snapfinger Creek	0.16	246	365	415	525
200 feet downstream of CSX Railroad	0.16	491	659	730	883

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH FORK PEACHTREE CREEK					
At the Fulton/DeKalb County boundary	28.27	9,906	15,436	16,032	23,602
Upstream of Briarcliff Road	20.43	7,704	11,896	12,346	18,054
Upstream of Southern Railway (CSX Transportation) bridge	18.13	7,995	12,266	12,725	18,584
At Homewood Court	10.94	5,236	7,939	8,227	11,873
Downstream of Valley Brook Road	7.63	3,889	5,967	6,190	9,001
Upstream of Stone Mountain Freeway	6.7	4,148	6,320	6,550	9,476
Approximately 3,000 feet downstream of Brockett Road	4.95	3,726	5,689	5,899	8,544
Upstream of Brockett Road	3.23	2,765	4,171	4,321	6,206
Upstream of South Fork Peachtree Creek Tributary B	2.36	2,000	2,968	3,071	4,360
At Harbour Oaks Road	1.79	1,975	2,851	2,943	4,094
Downstream of Elmdale Drive	0.81	1,163	1,708	1,765	2,493
At Hanfred Court	0.27	485	721	746	1,059
SOUTH FORK PEACHTREE CREEK TRIBUTARY					
At Desmond Drive	1.57	1,977	2,622	2,894	3,560
At Landover Drive	1.09	1,735	2,273	2,518	3,028
At Scott Boulevard	0.89	1,517	1,973	2,179	2,545
Just downstream of Church Street	0.72	1,472	2,117	2,377	3,214
At Church Street	0.56	1,148	1,652	1,855	2,500
Just upstream of confluence with South Fork Peachtree Creek Tributary Glendale Tributary	0.22	474	690	783	1,093

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH FORK PEACHTREE CREEK TRIBUTARY A					
At confluence with South Fork Peachtree Creek	0.84	926	1,328	1,484	2,165
Upstream of Jericho Road	0.35	405	626	650	950
SOUTH FORK PEACHTREE CREEK TRIBUTARY B					
At confluence with South Fork Peachtree Creek	0.87	865	1,338	1,389	2,028
Approximately 480 feet upstream of Oak Crest Road	0.54	632	985	1,023	1,501
SOUTH FORK PEACHTREE CREEK TRIBUTARY C					
At confluence with South Fork Peachtree Creek	2.01	2,013	2,928	3,024	4,229
Downstream of Mill Scott Street	1.38	1,866	2,671	2,756	3,809
SOUTH FORK PEACHTREE CREEK TRIBUTARY GLENDALE TRIBUTARY					
At mouth	0.16	339	456	495	607
Approximately 800 feet upstream of Glendale Avenue	0.16	372	546	621	870
SOUTH FORK PEACHTREE CREEK TRIBUTARY WILLOW TRIBUTARY					
At mouth	0.14	292	413	466	638

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER					
940 feet Upstream of Cross Section B	189.89	21,548	32,369	37,327	48,378
100 feet Upstream of confluence with Crooked Creek	188.64	21,548	32,374	37,331	48,381
500 feet Upstream of Cross Section G	184.64	21,382	32,125	37,045	48,011
200 feet Upstream of Confluence with Pole Bridge	182.20	22,988	34,720	40,055	51,685
2,100 feet Downstream of Cross Section K	160.41	20,676	31,547	36,531	47,264
2,000 feet Upstream of Cross Section L	159.17	21,053	32,254	37,379	48,420
500 feet Downstream of Cross Section Q	157.82	22,991	35,433	41,130	53,247
2,300 feet Upstream of Cross Section Q	152.62	22,781	35,082	40,711	52,672
100 feet Upstream of Confluence with South River Tributary 1	152.20	23,208	35,797	41,577	53,662
2,200 feet Upstream of Cross Section S	150.43	23,114	35,651	41,408	53,445
50 feet Downstream of Confluence with South River Tributary 2	150.03	23,427	36,076	41,877	53,990
200 feet Upstream of Cross Section W	148.72	24,298	37,598	43,648	56,084
500 feet Upstream of Confluence with South River Tributary 2	148.67	24,661	38,062	44,150	56,656
1,400 feet Downstream of Cross Section Z	147.26	24,607	37,976	44,050	56,524
70 feet Upstream of Confluence with Clark Creek	146.55	25,489	39,251	45,337	57,992
100 feet Downstream of Confluence with Snapfinger Creek	140.61	25,239	38,890	44,920	57,437
1,100 feet Downstream of Cross Section AE	102.15	17,726	27,629	32,100	41,221

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER (continued)					
At Confluence of South River Tributary 4	101.64	17,834	27,870	32,404	41,565
60 feet Upstream of Confluence of South River Tributary A	101.23	17,816	27,843	32,372	41,524
At Confluence of South River Tributary 5	99.55	17,944	28,160	32,746	41,955
600 feet Downstream of Cross Section AK	98.49	17,897	28,087	32,663	41,848
300 feet Upstream of Confluence with Conley Creek	98.26	18,042	28,383	32,962	42,203
1,900 feet Downstream of Confluence with South River Tributary 6	83.33	15,200	23,855	27,682	35,475
330 feet Upstream of Confluence with South River Tributary 6	82.87	15,363	24,280	28,090	36,133
60 feet Upstream of Confluence with South River Tributary 7	82.57	15,413	24,414	28,203	36,303
680 feet Upstream of Waldrop Road	82.37	15,403	24,399	28,186	36,277
500 feet Downstream of Cross Section AU	62.81	13,226	19,603	22,422	28,020
460 feet Upstream of Confluence with South River Tributary 8	62.45	13,377	19,748	22,584	28,171
1,200 feet Upstream of Cross Section AV	62.01	13,327	19,675	22,499	28,060
160 feet Upstream of Confluence with Doolittle Creek	59.34	13,528	19,830	22,573	27,908
1,860 feet Upstream of Cross Section BA	51.66	14,235	20,542	23,336	28,839
2,150 feet Downstream of Bouldercrest Road	42.43	11,939	17,080	19,435	23,777
270 feet Upstream of Bouldercrest Road	41.51	12,587	17,845	20,323	24,800

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER (continued)					
320 feet Upstream of confluence of South River Tributary 9	41.40	12,654	17,917	20,410	24,910
50 feet Upstream of Interstate 285	40.67	12,621	17,865	20,351	24,839
230 feet Downstream of Cross Section BN	39.97	13,995	20,139	22,922	28,845
350 feet Downstream of Cross Section BO	28.99	10,467	15,297	17,452	22,079
60 feet Downstream of Confluence of South River Tributary 12	28.23	10,209	14,957	17,085	21,666
740 feet Downstream of Cross Section BQ	26.61	9,972	14,615	16,708	21,216
At Confluence of South River Tributary 14	26.47	9,960	14,599	16,689	21,193
1,120 feet Upstream of Confluence of South River Tributary 14	26.22	9,931	14,557	16,642	21,133
SOUTH RIVER TRIBUTARY A					
600 feet Downstream of Cross Section B	1.22	572	840	912	1,212
30 feet Upstream of Lehigh Boulevard	1.07	546	806	875	1,161
1,400 feet Upstream of Lehigh Boulevard	1.07	1,047	1,570	1,775	2,220
40 feet Upstream of Flat Shoals Parkway	0.71	711	906	979	1,249
440 feet Upstream of Cross Section E	0.71	807	1,129	1,273	1,672
50 feet Upstream of Vera Cruz Drive	0.22	279	371	441	663
210 feet Upstream of Cross Section G	0.22	284	431	524	698
320 feet Upstream of Cheru Drive	0.17	257	387	443	567

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER					
TRIBUTARY 1					
740 feet Upstream of Confluence with South River	1.78	781	1,418	1,750	2,547
150 feet Upstream of Cross Section A	1.67	771	1,402	1,730	2,521
300 feet Upstream of Confluence of South River Tributary 1A	1.22	906	1,392	1,599	2,061
450 feet Upstream of Cross Section C	0.41	375	606	775	1,139
35 feet Upstream of Browns Mill Road	0.31	212	518	649	933
600 feet Downstream of Cross Section F	0.23	211	467	567	798
300 feet Downstream of Cross Section F	0.18	182	391	469	638
120 feet Upstream of Cross Section F	0.18	259	420	489	641
490 feet Upstream of Cross Section F	0.18	272	428	498	651
SOUTH RIVER					
TRIBUTARY 1A					
400 feet Downstream of Cross Section A	0.81	660	858	932	1,161
20 feet Upstream of Browns Mill Road	0.75	637	829	900	1,125
1,100 feet Upstream of Confluence with South River Tributary 1A	0.50	513	791	917	1,193
1,250 feet Upstream of Cross Section B	0.36	421	653	756	980
SOUTH RIVER					
TRIBUTARY 1AA					
1,300 feet Upstream of East Saddleridge Drive	0.23	219	357	419	557

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER					
TRIBUTARY 2					
165 feet Downstream of Cross Section A	1.31	827	1,177	1,265	1,447
30 feet Upstream of Panola Road	1.23	769	1,116	1,233	1,410
380 feet Upstream of Confluence with South River Tributary 2A	1.16	747	1,300	1,572	2,194
1,260 feet Downstream of Cross Section D	0.98	645	1,061	1,250	1,713
60 feet downstream of Confluence with South River Tributary 2B	0.72	424	873	1,048	1,411
70 feet Upstream of Wolverton Circle	0.43	290	619	725	868
1,100 feet downstream of Cross Section G	0.43	544	856	995	1301
1,100 feet Upstream of Cross Section F	0.21	259	411	478	626
SOUTH RIVER					
TRIBUTARY 2A					
70 feet Upstream of Cross Section B	0.18	109	245	324	522
100 feet Upstream of Cross Section C	0.18	237	388	455	607
SOUTH RIVER					
TRIBUTARY 2B					
750 feet Upstream of Confluence with South River Tributary 2	0.29	379	592	686	771
SOUTH RIVER					
TRIBUTARY 3					
40 feet Upstream of River Road	1.41	863	1,352	1,540	1,913
1,900 feet Upstream of River Road	1.16	897	1,470	1,729	2,306

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER					
TRIBUTARY 4					
260 feet Upstream of Cross Section B	0.25	290	462	538	707
SOUTH RIVER					
TRIBUTARY 5					
50 feet Downstream of Confluence with South River Tributary 5A	1.06	880	1,344	1,547	1,973
665 feet Downstream of Cross Section A	0.66	503	709	792	981
850 feet Upstream of Cross Section A	0.54	442	619	676	783
50 feet Upstream of River Road	0.44	405	573	625	722
1,400 feet Upstream of River Road	0.44	439	702	815	1,064
3,000 feet upstream of Cross Section C	0.24	252	391	452	587
SOUTH RIVER					
TRIBUTARY 5A					
450 feet Downstream of Cross Section B	0.41	454	713	824	1,057
600 feet Downstream of Cross Section C	0.23	274	384	431	581
60 feet Upstream of River Road	0.11	158	204	229	322
90 feet Upstream of River Road	0.11	163	246	282	360
SOUTH RIVER					
TRIBUTARY 6					
270 feet Upstream of Cross Section A	0.31	521	731	820	984
65 feet Upstream of Cherry Ridge Boulevard	0.20	334	474	526	673
515 feet Upstream of Cherry Ridge Boulevard	0.20	334	485	550	690

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER					
TRIBUTARY 7					
50 feet Upstream of Cross Section A	0.20	170	263	334	506
130 feet Upstream of Cross Section B	0.20	183	267	348	529
320 feet Upstream of Cross Section C	0.20	203	343	407	550
SOUTH RIVER					
TRIBUTARY 8					
600 feet Upstream of Cross Section A	0.44	250	366	408	494
100 feet Upstream of Columbia Drive	0.29	146	204	230	284
1,030 feet Upstream of Columbia Drive	0.29	668	937	1,053	1,301
SOUTH RIVER					
TRIBUTARY 9					
550 feet Upstream of Cross Section A	0.54	692	972	1,143	1,512
2,300 feet Upstream of Cross Section A	0.26	446	644	729	913
SOUTH RIVER					
TRIBUTARY 10					
630 feet Downstream of Cross Section C	0.35	525	761	855	1,046
230 feet Upstream of Cross Section C	0.16	277	386	425	499
400 feet Upstream of Cross Section C	0.16	284	405	457	569

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER					
TRIBUTARY 11					
780 feet Upstream of Confluence with South River	0.67	268	355	374	466
150 feet Upstream of Cross Section A	0.58	270	348	365	565
310 feet Downstream of Cross Section C	0.58	398	718	817	1,015
630 feet Downstream of Cross Section E	0.47	727	941	1,033	1,221
50 feet Upstream of Fleetwood Drive	0.27	413	486	509	554
600 feet Upstream of Fleetwood Drive	0.27	519	687	772	952
250 feet Downstream of Cross Section G	0.20	418	581	650	798
465 feet Upstream of Cross Section G	0.17	363	510	574	710
SOUTH RIVER					
TRIBUTARY 12					
560 feet Downstream of Cross Section B	1.31	1,325	1,640	1,739	1,918
170 feet Upstream of Interstate 675 Ramp	1.23	1,300	1,598	1,695	1,874
110 feet Upstream of Interstate 285 Ramp	1.15	1,200	1,552	1,695	1,952
400 feet Upstream of Henrico Road	0.82	833	1,064	1,164	1,378
170 feet Downstream of Confluence of South River Tributary 12A	0.76	793	1,007	1,100	1,302
940 feet Upstream of Confluence of South River Tributary 12A	0.40	627	828	916	1,107
155 feet Upstream of Cross Section F	0.28	449	630	708	853
90 feet Upstream of Thurman Drive	0.12	284	384	427	518

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SOUTH RIVER					
TRIBUTARY 12A					
300 feet Upstream of Cross Section A	0.36	172	187	193	202
560 feet Upstream of Cross Section A	0.36	678	967	1,092	1,359
SOUTH RIVER					
TRIBUTARY 14					
466 feet Downstream of Cross Section A	0.25	262	355	411	523
STEPHENSON CREEK					
At Cross Section B	3.49	1,258	1,623	1,755	2,016
At Browns Mill Road	3.14	1,213	1,563	1,686	1,921
At South Goddard Road	2.70	1,781	2,998	3,741	4,365
At Cross Section J	2.29	2,312	3,587	4,173	5,616
At Confluence with Tributary 2	1.68	1,536	2,455	2,873	3,941
Unnamed Road 400 feet Upstream of North Goddard Road	1.34	1,358	2,162	2,524	3,449
Pond 3,500 feet Upstream of North Goddard Road	0.88	1,248	1,976	2,302	3,014
Pond 300 feet Upstream of Rockland Road	0.64	1,016	1,560	1,807	2,351
At Cross Section U	0.20	339	506	581	747
STEPHENSON CREEK					
TRIBUTARY 1					
1,400 feet Upstream of Confluence with Stephenson Creek	0.61	811	1,189	1,357	1,723
500 feet Downstream of North Goddard Road	0.45	632	917	1,043	1,316
At North Goddard Road	0.21	269	397	453	578
At Cross Section G	0.16	214	312	356	451

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
STEPHENSON CREEK					
TRIBUTARY 2					
1,250 feet Upstream of Confluence with Stephenson Creek	0.34	518	779	898	1,164
2,400 feet Upstream of Confluence with Stephenson Creek	0.16	264	389	445	570
STONE MOUNTAIN CREEK					
At mouth	28.76	3,036	5,302	6,567	10,253
Above confluence with Wesley Branch	28.28	3,016	5,267	6,524	10,176
At confluence with Crooked Creek Tributary to Stone Mountain Creek	27.47	3,015	5,240	6,480	10,083
At dam just upstream of Rockbridge Road	18.45	1,859	3,256	3,987	6,010
Above confluence of Stone Mountain Creek Unnamed Tributary 1	18.14	3,481	5,777	6,882	9,790
Above confluence of Stone Mountain Creek Tributary A	16.22	2,103	3,480	4,156	5,957
Above confluence of Stone Mountain Creek Unnamed Tributary 2	15.68	1,577	2,562	3,061	4,429
Above confluence of Stone Mountain Creek Unnamed Tributary 3	15.16	1,532	2,502	2,994	4,319
Above confluence of Stone Mountain Creek Unnamed Tributary 4	14.83	1,502	2,455	2,940	4,245
Above confluence of Stone Mountain Creek Unnamed Tributary 5	14.61	1,488	2,436	2,920	4,218
At Stone Mountain Lake Dam	13.68	1,424	2,340	2,811	4,065
Above confluence of Stone Mountain Creek Unnamed Tributary 10	5.44	2,811	3,605	3,974	5,386
At US Highway-78	4.52	2,211	2,843	3,263	3,581

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	Drainage Area (<u>square miles</u>)	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent- Annual-Chance</u>	<u>2-Percent- Annual-Chance</u>	<u>1-Percent- Annual-Chance</u>	<u>0.2-Percent- Annual-Chance</u>
STONE MOUNTAIN CREEK (continued)					
Above confluence of Stone Mountain Creek Unnamed Tributary 12	4.16	2,916	3,884	4,302	5,579
Above confluence of Stone Mountain Creek Unnamed Tributary 14	3.72	2,549	3,320	3,651	4,472
Above confluence of Stone Mountain Creek Unnamed Tributary 15	2.75	2,048	2,552	2,765	3,287
Above confluence of Stone Mountain Creek Unnamed Tributary 16	2.38	1,775	2,047	2,151	2,411
Above confluence of Stone Mountain Creek Unnamed Tributary 17	2.03	2,294	3,418	3,922	5,511
Above confluence of Stone Mountain Creek Unnamed Tributary 18	1.79	1,976	2,960	3,404	4,937
Above confluence of Stone Mountain Creek Unnamed Tributary 19	1.31	1,743	2,590	2,972	3,920
Above confluence of Stone Mountain Creek Unnamed Tributary 20	0.80	907	1,362	1,569	2,086
Above confluence of Stone Mountain Creek Unnamed Tributary 21	0.51	807	1,188	1,358	1,777
STONE MOUNTAIN CREEK TRIBUTARY A					
At Waters Edge Drive	1.49	1,258	1,815	2,098	3,048

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
STONE MOUNTAIN CREEK UNNAMED TRIBUTARY 6					
At mouth	2.66	2,339	3,625	4,200	5,720
At dam approximately 3,800 feet downstream of Stonewall Jackson Drive	2.28	1,732	2,711	3,172	4,368
At dam approximately 2,100 feet downstream of Stonewall Jackson Drive	1.84	1,180	1,916	2,281	3,364
At Stonewall Jackson Drive	1.66	965	1,613	1,937	2,965
At dam approximately 1,000 feet upstream of Stonewall Jackson Drive	1.00	501	822	1,058	1,723
SUGAR CREEK					
At mouth	9.23	3,932	5,789	6,525	7,876
At Clifton Church Road	8.46	3,938	5,875	6,585	7,793
At Mary Lou Lane	7.39	5,863	6,993	7,497	8,521
At Brannen Road	7.10	5,820	6,906	7,315	8,164
At Interstate Route 20	6.13	5,401	6,300	6,603	7,379
At Fayetteville Road	5.10	4,988	5,874	6,156	7,218
At Glenwood Avenue	2.53	2,903	3,746	4,104	4,674
At Memorial Drive	1.47	1,654	2,561	3,064	3,454
At Whitefoord Avenue	0.19	400	552	616	755
SUGAR CREEK EAST BRANCH A					
At mouth	0.37	494	750	863	1,108

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SUGAR CREEK					
TRIBUTARY A					
At mouth	1.66	1,658	1,928	2,032	2,492
Just upstream of Glenwood Avenue	1.59	1,633	1,905	2,009	2,462
Just upstream of Lakeside Village Drive	1.24	1,385	1,809	2,044	2,604
At Boulevard Drive	0.85	1,207	1,725	1,948	2,428
At First Avenue	0.46	588	855	971	1,221
At Second Avenue	0.36	436	800	949	1,406
Just upstream of Second Ave	0.31	384	700	827	1,217
SUGAR CREEK					
TRIBUTARY A1					
At mouth	0.39	627	881	989	1,222
SUGAR CREEK					
TRIBUTARY B					
At mouth	0.25	364	541	618	784
SUGAR CREEK					
TRIBUTARY C					
At mouth	0.29	384	582	669	858
SUGAR CREEK					
TRIBUTARY D					
At mouth	0.23	305	448	510	645
SUGAR CREEK					
TRIBUTARY E					
At mouth	0.37	450	663	756	957
SUGAR CREEK					
TRIBUTARY F					
At mouth	0.17	277	406	462	583
SUGAR CREEK					
TRIBUTARY G					
At mouth	0.35	520	764	870	1,099

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SUGAR CREEK TRIBUTARY H At mouth	0.27	405	564	632	778
SUGAR CREEK TRIBUTARY I At mouth	0.56	999	1,385	1,551	1,905
At Boulevard Drive	0.33	583	806	902	1,106
SUGAR CREEK TRIBUTARY J At mouth	0.67	1,335	1,838	2,054	2,532
At Moreland Avenue	0.26	560	776	869	1,067
SUGAR CREEK TRIBUTARY J1 At mouth	0.29	603	834	933	1,145
SWIFT CREEK At mouth	7.57	2,048	3,555	4,308	5,932
At approximately 2,860 feet upstream of mouth	7.28	2,058	3,537	4,283	5,887
At approximately 4,800 feet downstream of Rock Chapel Road	6.61	2,020	3,379	4,055	5,527
Below confluence with Swift Creek Unnamed Tributary 1	6.51	2,766	3,378	4,043	5,513
Above confluence with Swift Creek Unnamed Tributary 1	5.90	2,630	3,218	3,879	5,292
At Rock Chapel Road	5.77	1,999	3,296	3,879	5,322
Above confluence with Swift Creek Tributary A	3.05	852	1,639	2,039	3,075
Above confluence with Swift Creek Unnamed Tributary 2	2.56	756	1,406	1,753	2,729
At approximately 1,950 feet downstream of Old Greystone Drive	2.23	731	1,350	1,673	2,577
At Old Greystone Drive	1.97	719	1,330	1,629	2,461
At approximately 2,500 feet upstream of Old Greystone Drive	1.41	650	1,045	1,297	2,068

TABLE 6 - SUMMARY OF DISCHARGES - CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
SWIFT CREEK (continued)					
At Rogers Lake Road	1.11	603	929	1,165	1,873
Above confluence with Swift Creek Unnamed Tributary 3	0.65	258	517	692	1,204
SWIFT CREEK TRIBUTARY A					
At mouth	2.72	1,240	2,149	2,628	3,795
Above confluence with Swift Creek Tributary A Unnamed Tributary 1	1.17	854	1,495	1,831	2,633
Above confluence with Swift Creek Tributary A Unnamed Tributary 2	0.41	558	858	990	1,305
At Rogers Lake Road	0.23	297	465	543	729
TOM GEORGE CREEK					
At County line	3.29	993	1,904	2,423	3,890
At dam approximately 950 feet upstream of county boundary	3.16	979	1,852	2,320	3,687
Below confluence with Tom George Creek Unnamed Tributary 1	2.97	1,088	1,835	2,292	3,468
Above confluence with Tom George Creek Unnamed Tributary 1	2.54	1,036	1,724	2,128	3,183

TABLE 6 - SUMMARY OF DISCHARGES – CONTINUED

<u>Flooding Source and Location</u>	<u>Drainage Area (square miles)</u>	<u>Peak Discharges (cubic feet per second)</u>			
		<u>10-Percent-Annual-Chance</u>	<u>2-Percent-Annual-Chance</u>	<u>1-Percent-Annual-Chance</u>	<u>0.2-Percent-Annual-Chance</u>
UNNAMED TRIBUTARY TO NANCY CREEK					
Downstream of East Brookhaven Drive	0.84	731	986	1,130	1,749
Upstream of Osborne Road	0.57	590	826	924	1,243
Upstream of Dorby Park Drive	0.52	648	862	912	1,206
Downstream of Hermance Drive	0.26	208	415	480	716
Downstream of Woodrow Way	0.15	23	25	25	27
UNNAMED TRIBUTARY 2 TO UNNAMED TRIBUTARY TO NANCY CREEK					
At mouth	0.20	276	446	514	684
Upstream of Fuller Road	0.18	329	497	546	709
At Cross Section E	0.08	139	209	230	299
YELLOW RIVER					
At Confluence with Lake Sorrento	223.00	11,195	19,916	24,840	40,055
At Pleasant Hill Road	216.00	10,998	19,516	24,312	39,049
Approximately 100 feet east of Norris Lake Drive	207.00	10,739	18,991	23,616	37,715
Approximately 550 feet downstream of Rockbridge Road	191.00	10,261	18,022	22,331	35,229
At Rockbridge Road	162.00	9,339	16,153	19,844	30,350

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the source studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the FIRM represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data tables in the FIS report. For construction and/or floodplain management purposes, users are encouraged to use the flood elevation data presented in this FIS in conjunction with the data shown on the FIRM.

Location of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway was computed (Section 4.2), selected cross-section locations are also shown on the FIRM (Exhibit 2).

Flood profiles were drawn showing computed water-surface elevations for floods of the selected recurrence intervals. Water-surface elevations of floods of the selected recurrence intervals were developed using the USACE HEC-2 step backwater computer program (USACE, 1991).

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the profiles are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

Precountywide Analyses

Each jurisdiction within DeKalb County, except the Cities of Stone Mountain, Avondale Estates, Lithonia, and Dunwoody, has a previously printed FIS report. The hydraulic analyses described in those reports have been compiled and are summarized below.

Cross sections of the backwater analysis for the Cities of Clarkston and Pine Lake, and the unincorporated areas of DeKalb County, were compiled by a combination of photogrammetric and field survey methods at selected intervals to model conveyance of the valleys, and field surveyed just upstream or downstream of bridges and culverts in order to compute the significant backwater effects of such structures.

Cross sections for the flooding sources located in the City of Atlanta were derived from Atlanta topographic maps (City of Atlanta, 1968 and 1981). The below-water sections were obtained by field measurement.

Cross section data for streams in the Cities of Chamblee, Decatur, and Doraville were obtained by field surveys. All bridges and culverts were surveyed to obtain elevation data and structural geometry.

Water-surface elevations of floods of the selected recurrence intervals were developed using the USACE HEC-2 step-backwater computer program (USACE, 1991).

The water-surface elevations in the City of Decatur were computed using the USACE HEC-2 step-backwater computer program (USACE, 1991). Cross-section geometry input to the program was obtained by field survey. All bridges, culverts, and retaining structures were field surveyed to obtain dimension and structural geometry. Starting water-surface elevations were determined using the slope/area method except where elevations were taken from previous FISs.

Water-surface profiles were developed for the Cities of Chamblee and Doraville using the USGS E431 step-backwater computer model. Embankment storage at all road crossings was investigated to determine the effect on the computed flood profiles. In the City of Doraville, Nancy Creek Tributary No. 1 at Tilly Mill Road, the effect was significant and the flood profiles were adjusted based on the reduction in peak discharge caused by storage.

In the City of Doraville, flood elevations for the upper end of the main tributaries and for the smallest tributaries were determined by approximate methods, based on regional stage-frequency drainage area relationships.

Starting water-surface elevations for the streams studied in detail in the City of Pine Lake were based on slope/area methods utilized in HEC-2 (USACE, 1991).

Starting water-surface elevations for the streams in the City of Clarkston were based on water-surface profiles developed for the entire South Fork Peachtree Creek watershed.

Channel roughness factors (Manning's "n") for these computations were assigned based on field inspection of the floodplain areas.

Roughness coefficients (Manning's "n") for all streams in the City of Chamblee were estimated by field inspection of stream reaches at each cross section. High watermarks for the September 1975 flood, obtained by USGS personnel, provided a known flood profile for use in the step-backwater analyses to verify or adjust the field roughness coefficients.

Channel roughness coefficients (Manning's "n") were based on field inspection in the Cities of Atlanta, Chamblee, Clarkston, Decatur, Doraville, Pine Lake, and the unincorporated areas of DeKalb County.

Embankment storage at road crossings in the City of Chamblee was investigated where necessary to determine the effect on computed flood profiles. Adjustments for reduction of peak discharges due to storage were made at the Chamblee Tucker Road crossings of North Fork Peachtree Creek Tributary B and Tributary

B-1, the Hickory Road crossing of North Fork Peachtree Creek Tributary A, and the Keswick Drive and Sexton Woods Drive crossings of Nancy Creek Tributary C.

May 7, 2001 Countywide FIS

Analyses of the hydraulic characteristics of the flooding sources were performed to provide estimations of water-surface elevations associated with the selected recurrence interval storms. Existing flood elevations were retained for adjoining stream sections when such information was available. Approximate methods were used to set flood elevations along intersecting streams not included in the restudy if existing data were not available for that stream.

The cross sections used for the analysis of the stream valleys were taken from a topographic map provided by the DeKalb County GIS Department. Extensive internal review and cross checks were performed by DeKalb County to ensure the accuracy and level of detail provided in the base map was in compliance with their internal requirements prior to the release of the data for use in this study. The use of this topographic base map was permitted through an inter-agency agreement between DeKalb County, Georgia, and FEMA, and the use of the mapping data is subject to that agreement.

The topographic map was generated from photogrammetric methods using 1995 aerial photographs. The methods used in the gathering of the photographic base data and the development of the associated topographic map were in compliance with the limitations set out in Flood Insurance Study Guidelines and Specifications for Study Contractors issued by FEMA in January 1995. Cross sections located upstream and downstream of bridges and other such impediments to flood routing were performed using field-survey techniques. These sections were collected using a local datum prior to the completion of the topographic base map and were then correlated to the provided base map upon its completion.

Channel roughness coefficients (Manning's "n" coefficient) were assigned based on field inspection and were guided through reference to the USGS Water-Supply Paper 1849 Roughness Characteristics of Natural Channels (U.S. Department of Interior, 1967). This reference describes channel characteristics associated with varying Manning's coefficients and provides photographs of these conditions for comparison to the existing field conditions.

The hydraulic analyses were performed using HEC-RAS version 2.1, as produced by the USACE. This software package was specifically used to perform the floodplain and floodway analyses. The floodplain was plotted on the topographic base map using the BOSS-HEC software package by importing the analyses prepared using the HEC-RAS software. Since the BOSS-HEC package interfaces directly with AutoCAD, the floodplains were exported directly onto the base topographic map. Thus, the accuracy of data transfer between the calculated

water-surface elevation and the plotted water-surface elevation was greatly improved over manual plotting techniques.

Starting water-surface elevations were developed based on the HEC-RAS slope/area calculation sub-routine.

Revision for May 16, 2013 Countywide FIS

The USACE's HEC-River Analysis System (HEC-RAS), Versions 3.1 – 4.1, were used to develop a one-dimensional steady flow hydraulic model of all streams in South River watershed and Brookhaven watershed (Tributary to Nancy Creek), up to the 100-acre drainage area limit. The HEC-RAS models were developed using the HEC-GeoRAS ArcMap interface.

DeKalb County's 2-foot interval topography map, or the 2010 Light Detection and Ranging (LiDAR) Terrain data were used to create a digital elevation model (DEM), which was used to extract cross sections and bridge/culvert sections in the studied watersheds. Cross sections were placed at critical hydraulic locations along the streams and near structures and extend beyond the maximum water surface elevation. The channel invert elevation of the cross sections was manually adjusted to correct the limitation of the DEM. To supplement the digital topographic data, field surveyed cross sections, dam, and road structure data were collected and incorporated in the hydraulic models.

The Manning's roughness coefficients were selected as referenced in the HEC-RAS Technical Reference Manual. The values were assigned based on the field survey photos and aerial imagery of the County. The contraction and expansion coefficients were used to model gradual contraction and expansion for all cross section. The entrance and exit loss coefficients, Manning's roughness coefficients, and weir coefficients for hydraulic structures were based on field survey photographs and sketches and used values recommended by the HEC-RAS User's Manual. Ineffective flow areas are coded in the studied models to account for areas adjacent to the floodplain which do not convey any flow. Ineffective flow areas are also coded upstream and downstream of hydraulic structures. Normal depth boundary conditions or known water-surface elevation boundary conditions were used in the studied watersheds.

Maximum water-surface elevations were computed for the existing 2-, 10-, 50-, 100-, and 500-year storm events. Floodway analyses were also performed for the existing condition detailed studied streams. The floodway is that portion of the available flow cross section that cannot be obstructed and cause an increase in the base flood water-surface elevations over a given amount. The floodway encroachments were set so that there is not an increase in the existing 100-year water-surface elevations of more than 1.0 feet at any cross section.

The channel and overbank "n" values for all the streams studied by detailed methods are shown in Table 7 "Manning's "n" Values." All revised data from the December 8, 2016, May 16, 2013, and May 7, 2001 FIS reports have been incorporated into the table.

TABLE 7 - MANNING'S "n" VALUES

<u>Stream</u>	<u>Channel "n"</u>	<u>Overbank "n"</u>
Barbashela Creek	0.020-0.100	0.017-0.100
Barbashela Creek Tributary A	0.018-0.100	0.018-0.100
Barbashela Creek Tributary B	0.018-0.100	0.045-0.100
Barbashela Creek Tributary C	0.045-0.090	0.018-0.100
Barbashela Creek Tributary D	0.030-0.100	0.010-0.100
Barbashela Creek Tributary D1	0.045-0.080	0.018-0.100
Barbashela Creek Tributary D2	0.045	0.018-0.100
Barbashela Creek Tributary E	0.030-0.045	0.018-0.100
Barbashela Creek Tributary E1	0.045-0.070	0.010-0.100
Barbashela Creek Tributary F	0.045-0.100	0.018-0.100
Barbashela Creek Tributary F1	0.045	0.018-0.100
Barbashela Creek Tributary G	0.045-0.100	0.018-0.100
Barbashela Creek Tributary H	0.030-0.045	0.018-0.100
Barbashela Creek Tributary H1	0.045	0.018-0.100
Barbashela Creek Tributary I	0.045	0.019-0.100
Barbashela Creek Tributary J	0.045	0.018-0.100
Blue Creek	0.018-0.048	0.030-0.110
Blue Creek Tributary 1	0.045	0.070-0.100
Blue Creek Tributary 2	0.030-0.045	0.060-0.110
Burnt Fork Creek	0.030-0.080	0.080-0.150
Camp Creek	0.045-0.100	0.030-0.120
Camp Creek Tributary A	0.040-0.100	0.030-0.100
Clarks Creek	0.045-0.115	0.045-0.150
Clarks Creek Tributary 1	0.045-0.115	0.045-0.115
Cobbs Creek	0.040-0.080	0.045-0.080
Cobbs Creek Tributary A	0.045	0.050-0.085
Cobbs Creek Tributary 1	0.045-0.080	0.045-0.080
Cobbs Creek Tributary 2	0.045	0.050-0.080
Cobbs Creek Tributary 3	0.045	0.040-0.080
Cobbs Creek Tributary 4	0.045	0.050-0.070
Cobbs Creek Tributary 5	0.045	0.050-0.080
Cobbs Creek Tributary 6	0.045	0.050-0.080
Cobbs Creek Tributary 7	0.045-0.080	0.050-0.080
Cobbs Creek Tributary 8	0.045	0.045-0.080
Conley Creek	0.040-0.120	0.035-0.120
Conley Creek Tributary 1	0.040-0.045	0.055-0.120
Conley Creek Tributary 2	0.040-0.050	0.060-0.120
Conley Creek Tributary 2A	0.040	0.060
Conley Creek Tributary 3	0.045-0.050	0.010-0.120
Conley Creek Tributary 3A	0.045-0.050	0.100-0.120
Conley Creek Tributary 4	0.040-0.120	0.040-0.120
Conley Creek Tributary 5	0.045	0.055-0.120
Conley Creek Tributary 5A	0.045	0.055-0.120
Conley Creek Tributary 6	0.043-0.050	0.065-0.120
Conley Creek Tributary 7	0.045-0.050	0.035-0.120

TABLE 7 - MANNING'S "n" VALUES - (continued)

<u>Stream</u>	<u>Channel "n"</u>	<u>Overbank "n"</u>
Corn Creek	0.045-0.050	0.045-0.150
Corn Creek Tributary 1	0.045-0.115	0.045-0.115
Corn Creek Tributary 2	0.045	0.050-0.115
Crooked Creek (South River)	0.045	0.055-0.090
Crooked Creek (South River) Tributary 1	0.045	0.045-0.080
Crooked Creek (South River) Tributary 2	0.045	0.055-0.080
Crooked Creek (South River) Tributary 3	0.045	0.055-0.080
Crooked Creek (South River) Tributary 4	0.045	0.065-0.080
Crooked Creek (South River) Tributary 5	0.045	0.065-0.080
Crooked Creek Tributary to Stone Mountain Creek	0.020-0.100	0.020-0.150
Crooked Creek Tributary A	0.030-0.100	0.020-0.100
Crooked Creek Tributary A-1	0.030-0.100	0.020-0.110
Doless Creek	0.050-0.120	0.013-0.120
Doolittle Creek	0.030-0.120	0.010-0.120
Doolittle Creek Branch A	0.050-0.080	0.010-0.120
Doolittle Creek East Branch B	0.050-0.120	0.050-0.120
Doolittle Creek Tributary A	0.035-0.120	0.050-0.120
Doolittle Creek Tributary B	0.050-0.080	0.005-0.120
Fowler Branch	0.045	0.045-0.080
Fowler Branch Tributary 1	0.045	0.050-0.100
Fowler Branch Tributary 2	0.045	0.050
Fowler Branch Tributary 3	0.045	0.050-0.080
Fowler Branch Tributary 4	0.013-0.045	0.013-0.080
Hardee Creek	0.050-0.055	0.050-0.100
Henderson Mill Creek	0.020-0.055	0.080-0.130
Honey Creek	0.013-0.050	0.020-0.120
Honey Creek Tributary A	0.030-0.050	0.030-0.120
Idle Creek	0.013-0.080	0.045-0.080
Indian Creek	0.045-0.080	0.018-0.110
Indian Creek Tributary A	0.045-0.070	0.018-0.080
Indian Creek Tributary B	0.045-0.055	0.018-0.100
Indian Creek Tributary C	0.045-0.050	0.018-0.100
Indian Creek Tributary D	0.050-0.060	0.018-0.110
Indian Creek Tributary D1	0.050	0.018-0.110
Indian Creek Tributary D2	0.045-0.050	0.018-0.100
Intrenchment Creek	0.042-0.048	0.018-0.100
Intrenchment Creek Tributary A	0.042-0.048	0.042-0.070
Intrenchment Creek Tributary 1	0.045	0.065-0.075
Intrenchment Creek Tributary 2	0.045-0.048	0.045-0.070
Intrenchment Creek Tributary 3	0.045	0.055-0.065
Intrenchment Creek Tributary 4	0.045-0.048	0.045-0.070
Johnson Creek	0.045-0.100	0.045-0.120
Little Stone Mountain Creek	0.030-0.110	0.030-0.110

TABLE 7 - MANNING'S "n" VALUES - (continued)

<u>Stream</u>	<u>Channel "n"</u>	<u>Overbank "n"</u>
Nancy Creek	0.020-0.065	0.070-0.130
Nancy Creek Tributary A	0.030-0.080	0.080-0.150
Nancy Creek Tributary B	0.030-0.120	0.080-0.150
Nancy Creek Tributary C	0.030-0.120	0.080-0.150
Nancy Creek Tributary C-1	0.030-0.120	0.080-0.150
Nancy Creek Tributary C-2	0.030-0.120	0.080-0.150
Nancy Creek Tributary D	0.030-0.080	0.080-0.150
Nancy Creek Tributary No. 1	0.020-0.120	0.040-0.200
Nancy Creek Tributary No. 1.1	0.065-0.120	0.040-0.200
Nancy Creek Tributary No. 1.2	0.065-0.120	0.040-0.200
Nancy Creek Tributary No. 2	0.065-0.120	0.040-0.200
North Fork Nancy Creek	0.030-0.080	0.080-0.150
North Fork Peachtree Creek	0.020-0.055	0.080-0.130
North Fork Peachtree Creek Tributary A	0.020-0.055	0.080-0.130
North Fork Peachtree Creek Tributary B	0.020-0.055	0.080-0.130
North Fork Peachtree Creek Tributary B-1	0.030-0.120	0.080-0.150
North Fork Peachtree Creek Tributary C	0.020-0.055	0.080-0.130
North Fork Peachtree Creek Tributary D-1	0.020-0.055	0.080-0.130
North Fork Peachtree Creek Tributary D-2	0.020-0.055	0.080-0.130
North Fork Peachtree Creek Tributary D-3	0.020-0.055	0.080-0.130
North Fork Peachtree Creek Tributary No. 1	0.065-0.120	0.040-0.200
North Fork Peachtree Creek Tributary No. 1.1	0.065-0.120	0.040-0.200
North Fork Peachtree Creek Tributary No. 2	0.065-0.120	0.040-0.200
Panthers Branch	0.045-0.120	0.045-0.120
Panthers Branch Tributary 1	0.030-0.045	0.060-0.120
Panthers Branch Tributary 2	0.045	0.060-0.120
Panthers Branch Tributary 3	0.045	0.120
Peachtree Branch	0.020-0.055	0.080-0.130
Peavine Creek	0.013-0.055	0.080-0.130
Peavine Creek Lamont Tributary	0.013-0.050	0.080-0.130
Peavine Creek Tributary	0.013-0.050	0.080-0.130
Peavine Creek Upland Tributary	0.013-0.050	0.080-0.130
Pine Mountain Creek	0.035-0.100	0.035-0.100
Plunkett Creek	0.030-0.050	0.060-0.120
Plunkett Creek Unnamed Tributary	0.030-0.050	0.030-0.120
Pole Bridge Creek	0.050-0.120	0.013-0.130
Pole Bridge Creek Tributary A	0.050-0.120	0.020-0.120
Pole Bridge Creek Tributary AA	0.050-0.080	0.050-0.120
Pole Bridge Creek Tributary AAA	0.050-0.120	0.080-0.120
Pole Bridge Creek Tributary B	0.050	0.013-0.120
Pole Bridge Creek Tributary BA	0.050	0.050-0.120
Pole Bridge Creek Tributary BAA	0.050	0.060-0.080
Pole Bridge Creek Tributary BB	0.050-0.120	0.030-0.120
Pole Bridge Creek Tributary BC	0.050-0.120	0.050-0.120
Pole Bridge Creek Tributary BD	0.050-0.120	0.018-0.120

TABLE 7 - MANNING'S "n" VALUES - (continued)

<u>Stream</u>	<u>Channel "n"</u>	<u>Overbank "n"</u>
Pole Bridge Creek Tributary C	0.050	0.120
Pole Bridge Creek Tributary D	0.050-0.120	0.050-0.120
Pole Bridge Creek Tributary E	0.050-0.120	0.050-0.120
Pole Bridge Creek Tributary F	0.050	0.050-0.120
Pole Bridge Creek Tributary FA	0.050	0.080-0.120
Pole Bridge Creek Tributary FB	0.050	0.080
Pole Bridge Creek Tributary G	0.050-0.120	0.050-0.120
Pole Bridge Creek Tributary H	0.050-0.120	0.013-0.120
Pole Bridge Creek Tributary HA	0.050	0.050-0.120
Pole Bridge Creek Tributary J	0.050-0.080	0.050-0.120
Pole Bridge Creek Tributary K	0.050-0.080	0.050-0.120
Pole Bridge Creek Tributary L	0.050	0.120
Pole Bridge Creek Tributary LA	0.050	0.120
Pole Bridge Creek Tributary M	0.080	0.080
Pole Bridge Creek Tributary N	0.050-0.120	0.050-0.120
Pole Bridge Creek Tributary O	0.050-0.080	0.050-0.080
Pole Bridge Creek Tributary P	0.050-0.120	0.013-0.120
Pole Bridge Creek Tributary Q	0.050-0.120	0.050-0.120
Shoal Creek	0.045-0.055	0.030-0.120
Shoal Creek Driftwood Tributary	0.013-0.045	0.050-0.080
Shoal Creek East Fork Middle Branch	0.045	0.050-0.120
Shoal Creek East Fork Middle Branch Tributary 1	0.045	0.050-0.120
Shoal Creek Kirk Tributary	0.045	0.035-0.120
Shoal Creek Tributary A	0.045	0.100-0.120
Shoal Creek Tributary 1	0.035-0.045	0.040-0.120
Shoal Creek Tributary 2	0.045	0.120
Shoal Creek Tributary 3	0.045	0.080
Shoal Creek Tributary 4	0.045	0.100
Shoal Creek Tributary 5	0.045	0.045-0.120
Shoal Creek Tributary 6	0.045	0.050-0.120
Shoal Creek Tributary 7	0.045	0.080-0.120
Shoal Creek Tributary 8	0.045	0.120
Shoal Creek Tributary 9	0.045	0.100-0.120
Shoal Creek West Tributary	0.013-0.050	0.080-0.100
Snapfinger Creek	0.045	0.013-0.110
Snapfinger Creek Tributary A	0.010-0.045	0.045-0.120
Snapfinger Creek Tributary A1	0.013-0.120	0.045-0.120
Snapfinger Creek Tributary B	0.045	0.045-0.120
Snapfinger Creek Tributary B1	0.045	0.045-0.120
Snapfinger Creek Tributary C	0.013-0.045	0.013-0.120
Snapfinger Creek Tributary C Split	0.010-0.045	0.045-0.070
Snapfinger Creek Tributary C1	0.045	0.030-0.120
Snapfinger Creek Tributary D	0.045	0.013-0.120
Snapfinger Creek Tributary E	0.045	0.060-0.120
Snapfinger Creek Tributary E1	0.045	0.070-0.080
Snapfinger Creek Tributary 1	0.010-0.045	0.060-0.120

TABLE 7 - MANNING'S "n" VALUES - (continued)

<u>Stream</u>	<u>Channel "n"</u>	<u>Overbank "n"</u>
Snapfinger Creek Tributary 2	0.045	0.080-0.120
Snapfinger Creek Tributary 3	0.010-0.045	0.030-0.120
Snapfinger Creek Tributary 4	0.045-0.100	0.045-0.120
Snapfinger Creek Tributary 5	0.045	0.045-0.120
Snapfinger Creek Tributary 6	0.018-0.045	0.045-0.120
Snapfinger Creek Tributary 7	0.013-0.045	0.045-0.120
Snapfinger Creek Tributary 8	0.045	0.045-0.120
Snapfinger Creek Tributary 9	0.010-0.045	0.045-0.120
Snapfinger Creek Tributary 9A	0.045	0.070-0.120
Snapfinger Creek Tributary 9B	0.013-0.045	0.045-0.120
Snapfinger Creek Tributary 10	0.045	0.120
Snapfinger Creek Tributary 11	0.045	0.080-0.120
Snapfinger Creek Tributary 12	0.010-0.045	0.045-0.120
Snapfinger Creek Tributary 13	0.045	0.070-0.120
Snapfinger Creek Tributary 14	0.045	0.030-0.120
Snapfinger Creek Tributary 15	0.045	0.030-0.120
Snapfinger Creek Tributary 16	0.045	0.060-0.100
Snapfinger Creek Tributary 17	0.010-0.045	0.050-0.120
South Fork Peachtree Creek	0.020-0.055	0.060-0.130
South Fork Peachtree Creek Tributary	0.013-0.050	0.080-0.100
South Fork Peachtree Creek Tributary A	0.020-0.055	0.060-0.130
South Fork Peachtree Creek Tributary B	0.020-0.055	0.060-0.130
South Fork Peachtree Creek Tributary C	0.020-0.055	0.060-0.130
South Fork Peachtree Creek Tributary Glendale Tributary	0.045	0.120
South Fork Peachtree Creek Tributary Willow Tributary	0.045	0.080
South River	0.030-0.115	0.045-0.150
South River Tributary A	0.013-0.095	0.045-0.115
South River Tributary 1	0.045-0.115	0.045-0.120
South River Tributary 1A	0.045-0.045	0.050-0.120
South River Tributary 1AA	0.045-0.045	0.035-0.115
South River Tributary 2	0.013-0.045	0.045-0.115
South River Tributary 2A	0.045-0.100	0.045-0.115
South River Tributary 2B	0.045-0.045	0.050-0.115
South River Tributary 3	0.045-0.115	0.050-0.115
South River Tributary 4	0.045-0.115	0.045-0.115
South River Tributary 5	0.045-0.045	0.050-0.115
South River Tributary 5A	0.045-0.050	0.045-0.120
South River Tributary 6	0.045-0.085	0.050-0.120
South River Tributary 7	0.045-0.045	0.050-0.115
South River Tributary 8	0.0435-0.115	0.035-0.120
South River Tributary 9	0.045-0.115	0.050-0.115
South River Tributary 10	0.045-0.045	0.045-0.115
South River Tributary 11	0.045-0.085	0.045-0.115

TABLE 7 - MANNING'S "n" VALUES - (continued)

<u>Stream</u>	<u>Channel "n"</u>	<u>Overbank "n"</u>
South River Tributary 12	0.013-0.115	0.045-0.120
South River Tributary 12A	0.045-0.115	0.050-0.115
South River Tributary 14	0.045-0.085	0.045-0.115
Stephenson Creek	0.045	0.055-0.100
Stephenson Creek Tributary 1	0.045	0.055-0.080
Stephenson Creek Tributary 2	0.045	0.055-0.080
Stone Mountain Creek	0.030-0.450	0.045-0.120
Stone Mountain Creek Tributary A	0.030-0.100	0.055-0.110
Stone Mountain Creek Unnamed Tributary 6	0.030-0.110	0.080-0.110
Sugar Creek	0.045	0.030-0.100
Sugar Creek East Branch A	0.055	0.055-0.120
Sugar Creek Tributary A	0.030-0.080	0.030-0.120
Sugar Creek Tributary A1	0.055	0.055-0.080
Sugar Creek Tributary B	0.055	0.060-0.120
Sugar Creek Tributary C	0.055	0.080-0.120
Sugar Creek Tributary D	0.055	0.055-0.080
Sugar Creek Tributary E	0.050	0.030-0.080
Sugar Creek Tributary F	0.055	0.030-0.080
Sugar Creek Tributary G	0.055	0.055-0.120
Sugar Creek Tributary H	0.050-0.080	0.030-0.080
Sugar Creek Tributary I	0.055	0.080
Sugar Creek Tributary J	0.055	0.080
Sugar Creek Tributary J1	0.055	0.060
Swift Creek	0.050-0.100	0.040-0.100
Swift Creek Tributary A	0.050-0.050	0.050-0.100
Tom George Creek	0.030-0.080	0.060-0.100
Unnamed Tributary to Nancy Creek	0.040-0.055	0.090-0.120
Unnamed Tributary 2 to Unnamed Tributary to Nancy Creek	0.055	0.100
Yellow River	0.035-0.060	0.055-0.120

No Manning roughness coefficients were listed for Lullwater Creek and Perimeter Creek in the May 7, 2001 countywide FIS.

3.3 Vertical Datum

All FISs and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum in use for newly created or revised FISs and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD29). With the finalization of the North American Vertical Datum of 1988 (NAVD88), many FIS reports and FIRMs are being prepared using NAVD88 as the referenced vertical datum.

All flood elevations shown in this FIS report and on the FIRM are referenced to NAVD88. Structure and ground elevations in the community must, therefore, be referenced to NAVD88. It is important to note that adjacent communities may be referenced to NGVD29. This may result in differences in base flood elevations across the corporate limits between the communities. The average conversion factor that was used to convert the data in this FIS report to NAVD88 was calculated using the National Geodetic Survey's VERTCON online utility (U.S. Department of Agriculture, 1973). The data points used to determine the conversion are listed in Table 8 "Vertical Datum Conversion".

Table 8 - Vertical Datum Conversion

<u>Quad Name</u>	<u>Corner</u>	<u>Latitude</u>	<u>Longitude</u>	<u>Conversion from NGVD29 to NAVD88</u>
Sandy Springs	SE	33.875	-84.375	0.230 feet
Chamblee	SE	33.875	-84.250	0.249 feet
Northwest Atlanta	SE	33.750	-84.375	0.269 feet
Northeast Atlanta	SE	33.750	-84.250	0.230 feet
Stone Mountain	SE	33.750	-84.125	0.141 feet
Snellville	SE	33.750	-84.000	0.082 feet
Southwest Atlanta	SE	33.625	-84.375	0.200 feet
Southeast Atlanta	SE	33.625	-84.250	0.174 feet
Redan	SE	33.625	-84.125	0.112 feet
AVERAGE				+0.187 feet

For more information on NAVD88, see the FEMA publication entitled *Converting the National Flood Insurance Program to the North American Vertical Datum of 1988* (FEMA, June 1992), or contact the Vertical Network Branch, National Geodetic Survey, Coast and Geodetic Survey, National Oceanic and Atmospheric Administration, Silver Spring, Maryland 20910 (Internet address <http://www.ngs.noaa.gov>).

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with the FIS report and FIRM for this community. Interested individuals may contact FEMA to access these data.

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. To assist in this endeavor, each FIS provides 100-year floodplain data, which may include a combination of the following: 10-, 50-, 100-, and 500-year flood elevations; delineations of the 100-year and 500-year floodplains; and 100-year floodway. This information is presented on the FIRM and in many components of the FIS, including Flood Profiles, Floodway Data tables, and Summary of Stillwater Elevation tables. Users should reference the data presented in the FIS as well as additional information that may be available at the local community map repository before making flood elevation and/or floodplain boundary determinations.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent-annual-chance (100-year) flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent-annual-chance (500-year) flood is employed to indicate additional areas of flood risk in the county. For the streams studied in detail, the 100- and 500-year floodplains have been delineated using the flood elevations determined at each cross section. For this FIS revision, the floodplain boundaries of the detailed studied streams listed in Table 3A were delineated on the latest DeKalb County 2010 LiDAR Terrain data using ArcGIS floodplain delineation toolset. For the other detailed studies (excluded on Table 3A), a datum conversion of 0.2 foot has been added to the flood elevations determined at each cross section and re-delineated on the latest DeKalb County 2010 LiDAR Terrain data using the ArcGIS floodplain delineation toolset.

For streams studied by approximate methods, the 100-year floodplain boundaries were delineated on the latest DeKalb County 2010 LiDAR Terrain data. Some portion of the approximate 100-year floodplain boundary was also taken from DeKalb County estimated approximate floodplain delineation.

The 100- and 500-year floodplain boundaries are shown on the FIRM (Exhibit 2). On this map, the 100-year floodplain boundary corresponds to the boundary of the areas of special flood hazards (Zones A and AE); and the 500-year floodplain boundary corresponds to the boundary of areas of moderate flood hazards. In cases where the 100- and 500-year floodplain boundaries are close together, only the 100-year floodplain boundary has been shown. Small areas within the

floodplain boundaries may lie above the flood elevations but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

For the streams studied by approximate methods, only the 100-year floodplain boundary is shown on the FIRM (Exhibit 2).

4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 100-year floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas that must be kept free of encroachment so that the 100-year flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1.0 foot, provided that hazardous velocities are not produced. The floodways in this study are presented to local agencies as a minimum standard that can be adopted directly or that can be used as a basis for additional floodway studies.

The floodways presented in this study were computed for certain stream segments on the basis of equal conveyance reduction from each side of the floodplain. Floodway widths were computed at cross sections. Between cross sections, the floodway boundaries were interpolated. The results of the floodway computations are tabulated for selected cross sections (Table 10) located in Volume 2. The computed floodway is shown on the FIRM (Exhibit 2). In cases where the floodway and 100-year floodplain boundaries are either close together or collinear, only the floodway boundary is shown.

Near the mouths of streams studied in detail, floodway computations are made without regard to flood elevations on the receiving water body. Therefore, "Without Floodway" elevations presented in Table 10, located in Volume 2, was taken from the detailed studied streams without considering the backwater effects from the receiving stream.

Encroachment into areas subject to inundation by floodwaters having hazardous velocities aggravates the risk of flood damage, and heightens potential flood hazards by further increasing velocities. A listing of stream velocities at selected cross sections is provided in Table 10 "Floodway Data" located in Volume 2. To reduce the risk of property damage in areas where the stream velocities are high, the community may wish to restrict development in the areas outside the floodway.

Burnt Fork Creek, Nancy Creek Tributary D, North Fork Nancy Creek, and portions Nancy Creek Tributary B and Nancy Creek Tributary C do not have floodways.

The area between the floodway and 100-year floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water surface elevation of the 100-year flood more than 1 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 2.

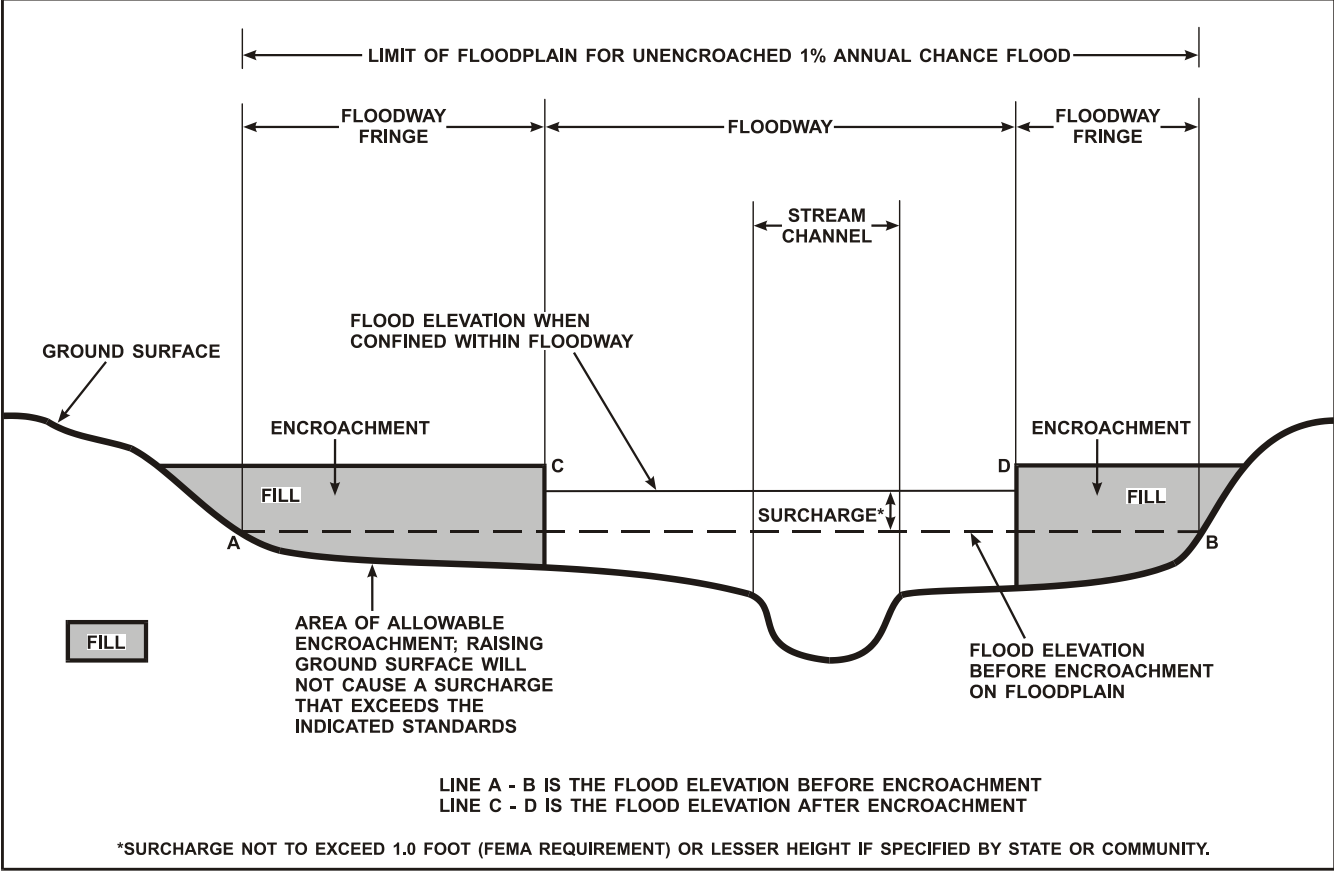


Figure 2 - Floodway Schematic

5.0 INSURANCE APPLICATIONS

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. These zones are as follows:

Zone A

Zone A is the flood insurance risk zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no BFEs or base flood depths are shown within this zone.

Zone AE

Zone AE is the flood insurance risk zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by detailed methods. In most instances, whole-foot BFEs derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone AH

Zone AH is the flood insurance rate zone that corresponds to the areas of 100-year shallow flooding (usually areas of ponding) where average depths are between 1 and 3 feet. Whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone AO

Zone AO is the flood insurance rate zone that corresponds to the areas of 100-year shallow flooding (usually sheet flow on sloping terrain) where average depths are between 1 and 3 feet. Average whole-foot depths derived from the detailed hydraulic analyses are shown within this zone.

Zone A99

Zone A99 is the flood insurance rate zone that corresponds to areas of the 100-year floodplain that will be protected by a Federal flood protection system where construction has reached specified statutory milestones. No base flood elevations or depths are shown within this zone.

Zone X

Zone X is the flood insurance risk zone that corresponds to areas outside the 500-year floodplain, areas within the existing-conditions 500-year floodplain, areas between the existing-conditions and future-conditions 100-year floodplain boundaries, and to areas of 100-year flooding where average depths are less than 1 foot, areas of 100-year flooding where the contributing drainage area is less than 1 square mile, and areas protected from the 100-year flood by levees. No BFEs or base flood depths are shown within this zone.

6.0 FLOOD INSURANCE RATE MAP

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance risk zones as described in Section 5.0 and, in the existing-conditions 100-year floodplains that were studied by detailed methods, shows selected whole-foot BFEs or average depths. Insurance agents use the zones and BFEs for existing-conditions in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by cross-hatching, tints, screens, and symbols, the 100-year and 500-year floodplains, floodways, and the locations of selected cross sections used in the hydraulic analyses and floodway computations.

The countywide FIRM presents flooding information for the entire geographic area of DeKalb County. Previously, FIRMs were prepared for each incorporated community and the unincorporated areas of the County identified as flood-prone. This countywide FIRM also includes flood-hazard information that was presented separately on Flood Boundary and Floodway Maps, where applicable. Historical data relating to the maps prepared for each community are presented in Table 9, "Community Map History."

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE DATE	FIRM REVISIONS DATE
Atlanta, City of	October 14, 1971	None	October 14, 1971	July 1, 1974 November 12, 1976 January 3, 1985 March 4, 1987
Avondale Estates, City of ¹ Brookhaven, City of ²	None June 2, 1970	None April 15, 1977	None May 15, 1980	None July 5, 1983 July 17, 1986 September 2, 1988 December 5, 1989 June 15, 1994
Chamblee, City of Clarkston, City of Decatur, City of	June 7, 1974 February 21, 1975 June 16, 1970	January 23, 1976 None None	September 15, 1977 June 15, 1981 June 19, 1970	None April 17, 1984 July 1, 1974 November 28, 1975 August 3, 1992
DeKalb County (Unincorporated Areas)	June 2, 1970	April 15, 1977	May 15, 1980	July 5, 1983 July 17, 1986 September 2, 1988 December 5, 1989 June 15, 1994
Doraville, City of Dunwoody, City of ²	June 7, 1974 June 2, 1970	January 23, 1976 April 15, 1977	September 1, 1977 May 15, 1980	August 5, 1985 July 5, 1983 July 17, 1986 September 2, 1988 December 5, 1989 June 15, 1994
Lithonia, City of ¹ Pine Lake, City of Stone Mountain, City of	None April 12, 1974 April 12, 1974	None April 16, 1976 July 9, 1976	None June 15, 1981 August 1, 1986	None None None

¹This community did not have a FIRM prior to the first countywide FIRM for DeKalb County

²This community's dates are taken from DeKalb County

TABLE 9

**FEDERAL EMERGENCY MANAGEMENT AGENCY
DEKALB COUNTY, GA
AND INCORPORATED AREAS**

COMMUNITY MAP HISTORY

7.0 OTHER STUDIES

This report either supersedes or is compatible with all previous studies on streams studied in this report and should be considered authoritative for purposes of the NFIP.

This is a multi-volume FIS. Each volume may be revised separately, in which case it supersedes the previously printed volume. Users should refer to the Table of Contents in Volume 1 for the current effective date of each volume; volumes bearing these dates contain the most up-to-date flood hazard data.

8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting FEMA, Federal Insurance and Mitigation Division, Federal Regional Center, 3003 Chamblee Tucker Road, Atlanta, Georgia 30341.

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10.0 REVISIONS DESCRIPTIONS

This section has been added to provide information regarding significant revisions made since the original FIS report and FIRM were printed. Future revisions may be made that do not result in the republishing of the FIS report. All users are advised to contact the Community Map Repository to obtain the most up-to-date flood hazard data.

10.1 First Revision (Revised December 8, 2016)

a. Acknowledgements

The hydrologic and hydraulic analyses for this restudy were performed by Dewberry Consultants, LLC. Local detailed studies were also leveraged and updated. The analyses covered the Upper Ocmulgee Watershed portion of DeKalb County. This work was completed under contract number EMA-2011-CA-5144 on October 2013.

Base map information shown on this FIRM was provided in digital format by DeKalb County aerial photography produced at a scale of 1:1200 from photography dated 2010 or later.

b. Coordination

A Discovery meeting was held with representatives from FEMA, the community, and the study contractor to explain the nature and purpose of the study area. An initial Consultation Coordination Officer's (CCO) meeting was also held to explain the nature and purpose of the FIS, and to identify the streams to be studied or restudied. A final CCO meeting was held with FEMA, the community, and the study contractor to review the results of the study.

The Discovery meeting was held on March 5, 2012 and attended by local officials, FEMA and Georgia DNR. The initial CCO meeting was held on February 13, 2013 and the final CCO meeting was held on March 23, 2015.

c. Scope

This revision incorporates revised analyses and mapping for Johnson Creek, Stone Mountain Creek, Yellow River and their respective tributaries. Local detailed studies were also leveraged and updated.

At the time of this revision, no Letters of Map Revision (LOMRs) were incorporated for the revised panels.

As part of this revision, updated analyses were included for the following sources:

<u>Stream</u>	<u>Limits of Revised Study</u>
Camp Creek	From county boundary to approximately 3500 feet downstream of Mountain Industrial Boulevard.
Camp Creek Tributary A	From the confluence with Camp Creek to approximately 388 feet downstream of Leeshire Road.
Crooked Creek Tributary A	From the confluence with Crooked Creek Tributary to Stone Mountain Creek to approximately 27 feet downstream of Shadow Rock Drive.
Crooked Creek Tributary A-1	From the confluence with Crooked Creek Tributary A to approximately 1600 feet upstream of Shadow Rock Drive.
Crooked Creek Tributary to Stone Mountain Creek	From the confluence with Stone Mountain Creek to a dam downstream of Longbow Drive.
Johnson Creek	From the confluence with Yellow River to approximately 2500 feet upstream of Johnson Creek Drive.
Pine Mountain Creek	From the confluence with Tom George Creek to approximately 1267 feet upstream of Turner Hill Road.
Little Stone Mountain Creek	From the confluence with Stone Mountain Creek to approximately 113 feet downstream of Skytop Drive.
Stone Mountain Creek	From the confluence with Yellow River to approximately 328 feet upstream of confluence with Stone Mountain Creek Unnamed Tributary 21.

<u>Stream</u>	<u>Limits of Revised Study</u>
Stone Mountain Creek Tributary A	From the confluence with Stone Mountain Creek to approximately 1207 feet upstream of Deshon Road.
Stone Mountain Creek Unnamed Tributary 6	From the confluence with Stone Mountain Creek to approximately 947 feet upstream of Stonewall Jackson Drive.
Swift Creek	From the confluence with Yellow River to approximately 2875 feet upstream of Regal Heights Drive.
Swift Creek Tributary A	From the confluence with Swift Creek to approximately 30 feet upstream of Rogers Lake Road.
Tom George Creek	From the DeKalb County Boundary to the confluence with Pine Mountain Creek.
Yellow River	From the DeKalb County Boundary to Rockbridge Road/ DeKalb County Boundary.

d. Hydrologic and Hydraulic Analyses

New hydrologic and hydraulic analyses were prepared for Camp Creek, Camp Creek Tributary A, Crooked Creek Tributary to Stone Mountain Creek, Crooked Creek Tributary A, Crooked Creek Tributary A-1, Pine Mountain Creek, Johnson Creek, Little Stone Mountain Creek, Stone Mountain Creek, Stone Mountain Creek Tributary A, Swift Creek, Swift Creek Tributary A, Swift Creek Unnamed Tributary 3, Tom George Creek and Yellow River. The detail hydrology and hydraulics were performed for 20-, 10-, 4-, 2-, 1-, 0.2-percent-annual-chance flood events. The hydrology and hydraulics parameters were based DeKalb County's 2-foot interval topography map, or the 2010 Light Detection and Ranging (LiDAR) Terrain data.

The Hydrologic Engineering Center (HEC) computer software HEC-HMS (version 3.5) was used to model rainfall-runoff for the new detailed study watersheds except Yellow River which was based on gage analysis.

The NRCS unit hydrograph transform method was used for the studies. The time of concentration for each sub-basin was calculated following the methodology described by the NRCS Technical Release 55 (TR-55), June 1986. The longest flow path for each sub-basin was delineated and manually sub-divided to represent sheet, shallow concentrated and channel flow to calculate the total time of concentration and lag time, applying the appropriate equations and parameters per the TR-55 guidelines.

The NRCS Curve Number (CN) methodology was selected to estimate the losses from a precipitation event occurring over the study stream watersheds. Composite curve numbers for each sub-basin were calculated based on the area computed for each land use in each soil type. Rainfall losses were estimated based on soil type and land use land cover properties for the watersheds.

Frequency storm method was selected to model the precipitation over all the streams listed above except the Yellow River. The 24-hour rainfall depths were taken from the NOAA Atlas 14 Volume 9; Version 2 (NOAA, 2013).

For Yellow River, gage analyses were performed using the U.S. G. S. Annual Peak Flow Frequency Analysis program (PKFQWin Ver. 5.2). Gage flow transfer from one gage to another was done to increase the confidence of the result obtained from PKFQwin analysis.

The Hydrologic Engineering Center (HEC) computer software HEC-RAS (version 4.1.0) was used to analyses and determine water surface elevations 20-, 10-, 4-, 2-, 1-, 0.2-percent-annual-chance (5, 10, 25, 50, 100, 500-yr, respectively) flood events and floodway for the study streams.

DeKalb County's 2-foot interval topography map, or the 2010 (LiDAR) Terrain data were used to create a digital elevation model (DEM), which was used to extract cross sections and bridge/culvert sections in the studied watersheds. Cross sections were placed at critical hydraulic locations along the streams and near structures and extend beyond the maximum water surface elevation. The channel invert elevation of the cross sections was adjusted to correct the limitation of the DEM. To supplement the digital topographic data, field surveyed cross sections, dam, and road structure data were collected and incorporated in the hydraulic models.

The Manning's roughness coefficients were selected as referenced in the HEC-RAS Technical Reference Manual. The values were assigned based on the field survey photos and aerial

imagery of the County. The contraction and expansion coefficients were used to model gradual contraction and expansion for all cross section. The entrance and exit loss coefficients, Manning's roughness coefficients, and weir coefficients for hydraulic structures were based on field survey photographs and sketches and used values recommended by the HEC-RAS User's Manual. Ineffective flow areas are coded in the studied models to account for areas adjacent to the floodplain which do not convey any flow. Ineffective flow areas are also coded upstream and downstream of hydraulic structures. Normal depth boundary conditions or known water-surface elevation boundary conditions were used in the studied watersheds.

Floodway analyses were also performed for the existing condition detailed studied streams. The floodway is that portion of the available flow cross section that cannot be obstructed and cause an increase in the base flood water-surface elevations over a given amount. The floodway encroachments were set so that there is not an increase in the existing 100-year water-surface elevations of more than 1.0 feet at any cross section.

e. Other Studies

Revisions for Rockdale County and Walton County are in progress as of the date of this revision. The purpose of the revisions is to update studies identified under the FY12 Risk MAP project.

f. Bibliography and References

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g. Considerations

Community boundaries were updated to reflect the newly annexed City of Brookhaven. Although the City of Brookhaven is not impacted by this revision, the FIS and map index have been updated to reflect the new community.

This revision to the DeKalb countywide FIS incorporates new analysis and mapping information for FIRM panels revised on December 8, 2016. This section presents important considerations for using the information contained in this FIS Report and in the FIRM panels updated by this revision. These considerations include changes in format and content. Figures 2 and 3 present information that applies to using the updated FIRM panels with the FIS Report.

Figure 3 - FIRM Notes to Users

NOTES TO USERS

For information and questions about this map, available products associated with this FIRM including historic versions of this FIRM, how to order products, or the National Flood Insurance Program in general, please call the FEMA Map Information eXchange at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA Map Service Center website at <http://msc.fema.gov>. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website. Users may determine the current map date for each FIRM panel by visiting the FEMA Map Service Center website or by calling the FEMA Map Information eXchange.

Communities annexing land on adjacent FIRM panels must obtain a current copy of the adjacent panel as well as the current FIRM Index. These may be ordered directly from the Map Service Center at the number listed above.

To determine if flood insurance is available in the community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

The map is for use in administering the NFIP. It may not identify all areas subject to flooding, particularly from local drainage sources of small size. Consult the community map repository to find updated or additional flood hazard information.

BASE FLOOD ELEVATIONS: For more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables within this FIS Report. Use the flood elevation data within the FIS Report in conjunction with the FIRM for construction and/or floodplain management.

FLOODWAY INFORMATION: Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the FIS Report for this jurisdiction.

FLOOD CONTROL STRUCTURE INFORMATION: Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of this FIS Report for information on flood control structures for this jurisdiction.

Figure 3 - FIRM Notes to Users

PROJECTION INFORMATION: The projection used in the preparation of the map was State Plane Transverse Mercator, Georgia West Zone (FIPS Zone 1002). The horizontal datum was North American Datum of 1983 (NAD83), GRS1980 spheroid. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of the FIRM.

ELEVATION DATUM: Flood elevations on the FIRM are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

*NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, Maryland 20910-3282
(301) 713-3242*

Local vertical monuments may have been used to create the map. To obtain current monument information, please contact the appropriate local community listed on the separately published FIRM Index.

BASE MAP INFORMATION: Base map information shown on the FIRM was provided by DeKalb County aerial photography produced at a scale of 1:1200 from photography dated 2010 or later.

Corporate limits shown on the map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after the map was published, map users should contact appropriate community officials to verify current corporate limit locations.

NOTES FOR FIRM INDEX

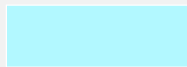
REVISIONS TO INDEX: As new studies are performed and FIRM panels are updated within DeKalb County, Georgia, corresponding revisions to the FIRM Index will be incorporated within the FIS Report to reflect the effective dates of those panels. Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located. The most recent FIRM panel effective date will correspond to the most recent index date.

SPECIAL NOTES FOR SPECIFIC FIRM PANELS

This Notes to Users section was created specifically for FIRM panels revised on December 8, 2016 in the DeKalb County, Georgia countywide Flood Insurance Study.

Figure 4 - Map Legend for FIRM

SPECIAL FLOOD HAZARD AREAS: *The 1% annual chance flood, also known as the base flood or 100-year flood, has a 1% chance of happening or being exceeded each year. Special Flood Hazard Areas are subject to flooding by the 1% annual chance flood. The Base Flood Elevation is the water surface elevation of the 1% annual chance flood. The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights. See note for specific types. If the floodway is too narrow to be shown, a note is shown.*



Special Flood Hazard Areas subject to inundation by the 1% annual chance flood (Zones A, AE, AH, AO, AR, A99, V and VE)

- Zone A The flood insurance rate zone that corresponds to the 1% annual chance floodplains. No base (1% annual chance) flood elevations (BFEs) or depths are shown within this zone.
- Zone AE The flood insurance rate zone that corresponds to the 1% annual chance floodplains. Base flood elevations derived from the hydraulic analyses are shown within this zone, either at cross section locations or as static whole-foot elevations that apply throughout the zone.
- Zone AH The flood insurance rate zone that corresponds to the areas of 1% annual chance shallow flooding (usually areas of ponding) where average depths are between 1 and 3 feet. Whole-foot BFEs derived from the hydraulic analyses are shown at selected intervals within this zone.
- Zone AO The flood insurance rate zone that corresponds to the areas of 1% annual chance shallow flooding (usually sheet flow on sloping terrain) where average depths are between 1 and 3 feet. Average whole-foot depths derived from the hydraulic analyses are shown within this zone.
- Zone AR The flood insurance rate zone that corresponds to areas that were formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- Zone A99 The flood insurance rate zone that corresponds to areas of the 1% annual chance floodplain that will be protected by a Federal flood protection system where construction has reached specified statutory milestones. No base flood elevations or flood depths are shown within this zone.
- Zone V The flood insurance rate zone that corresponds to the 1% annual chance coastal floodplains that have additional hazards associated with storm waves. Base flood elevations are not shown within this zone.
- Zone VE Zone VE is the flood insurance rate zone that corresponds to the 1% annual chance coastal floodplains that have additional hazards associated with storm waves. Base flood elevations derived from the coastal analyses are shown within this zone as static whole-foot elevations that apply throughout the zone.



Regulatory Floodway determined in Zone AE.

Figure 4 - Map Legend for FIRM

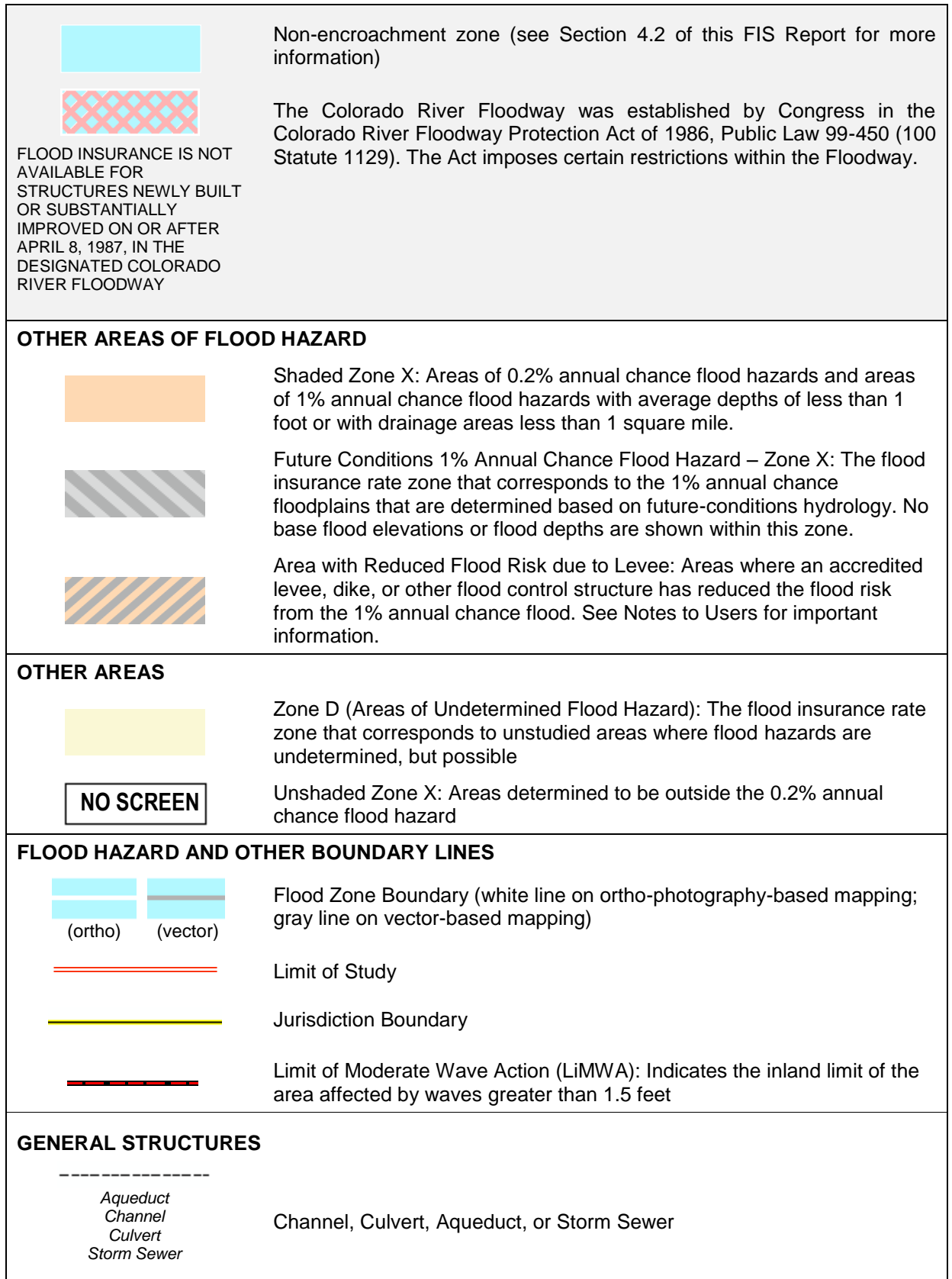


Figure 4 - Map Legend for FIRM

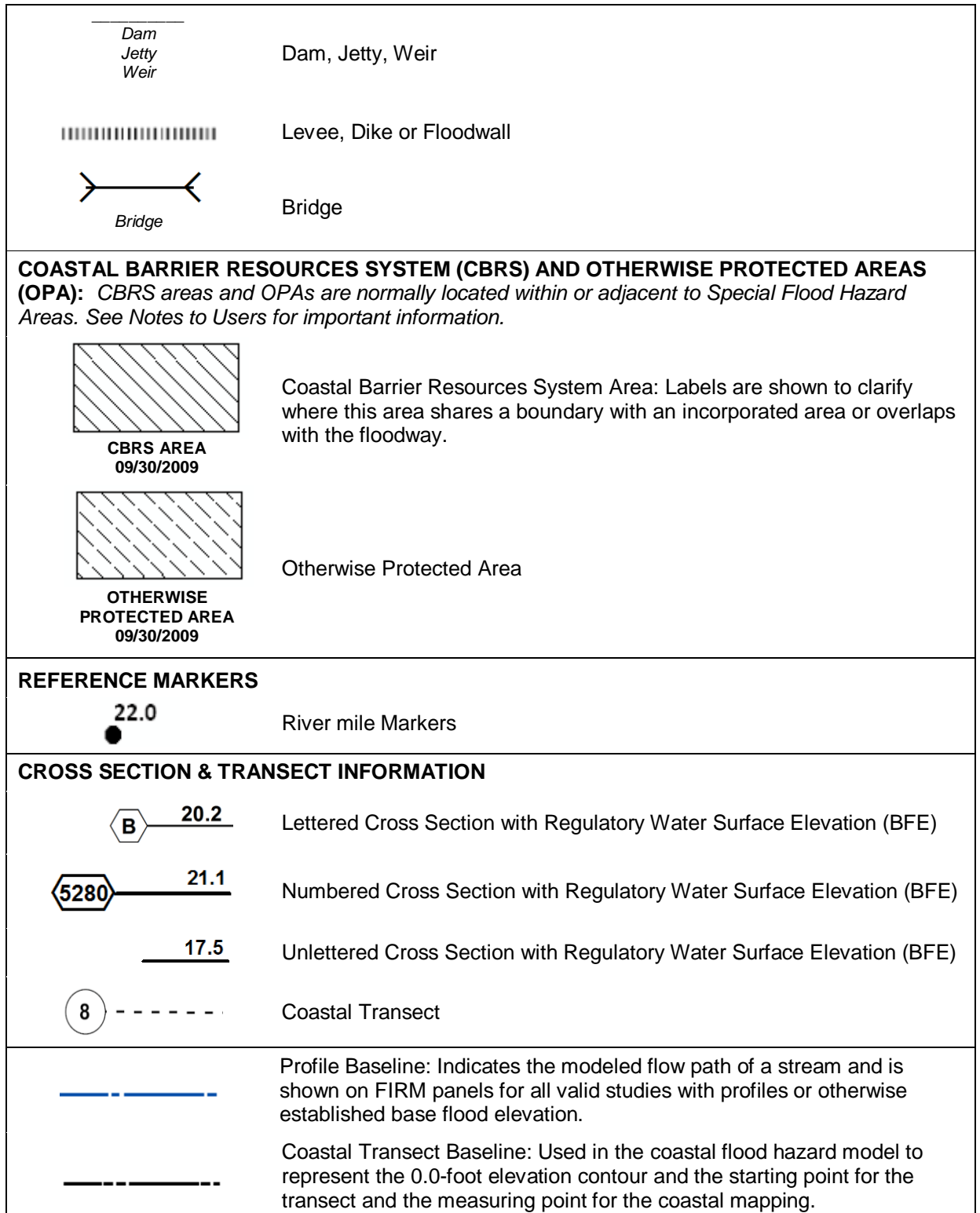
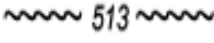










Figure 4 - Map Legend for FIRM

	Base Flood Elevation Line (shown for flooding sources for which no cross sections or profile are available)
ZONE AE (EL 16)	Static Base Flood Elevation value (shown under zone label)
ZONE AO (DEPTH 2)	Zone designation with Depth
ZONE AO (DEPTH 2) (VEL 15 FPS)	Zone designation with Depth and Velocity
BASE MAP FEATURES	
<u>Missouri Creek</u>	River, Stream or Other Hydrographic Feature
	Interstate Highway
	U.S. Highway
	State Highway
	County Highway
<u>MAPLE LANE</u>	Street, Road, Avenue Name, or Private Drive if shown on Flood Profile
 RAILROAD	Railroad
	Horizontal Reference Grid Line
	Horizontal Reference Grid Ticks
	Secondary Grid Crosshairs
Land Grant	Name of Land Grant
7	Section Number
R. 43 W. T. 22 N.	Range, Township Number
4276⁰⁰⁰mE	Horizontal Reference Grid Coordinates (UTM)
365000 FT	Horizontal Reference Grid Coordinates (State Plane)
80° 16' 52.5"	Corner Coordinates (Latitude, Longitude)